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VAMC SALISBURY
BUILDING 11-ADDITIONS
& RENOVATION

City, Stote Salisbury, NC

VA Contract Number VA246-12-C-0089

Issued For

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Sheet Title

PARTIAL GROUND FLOOR /

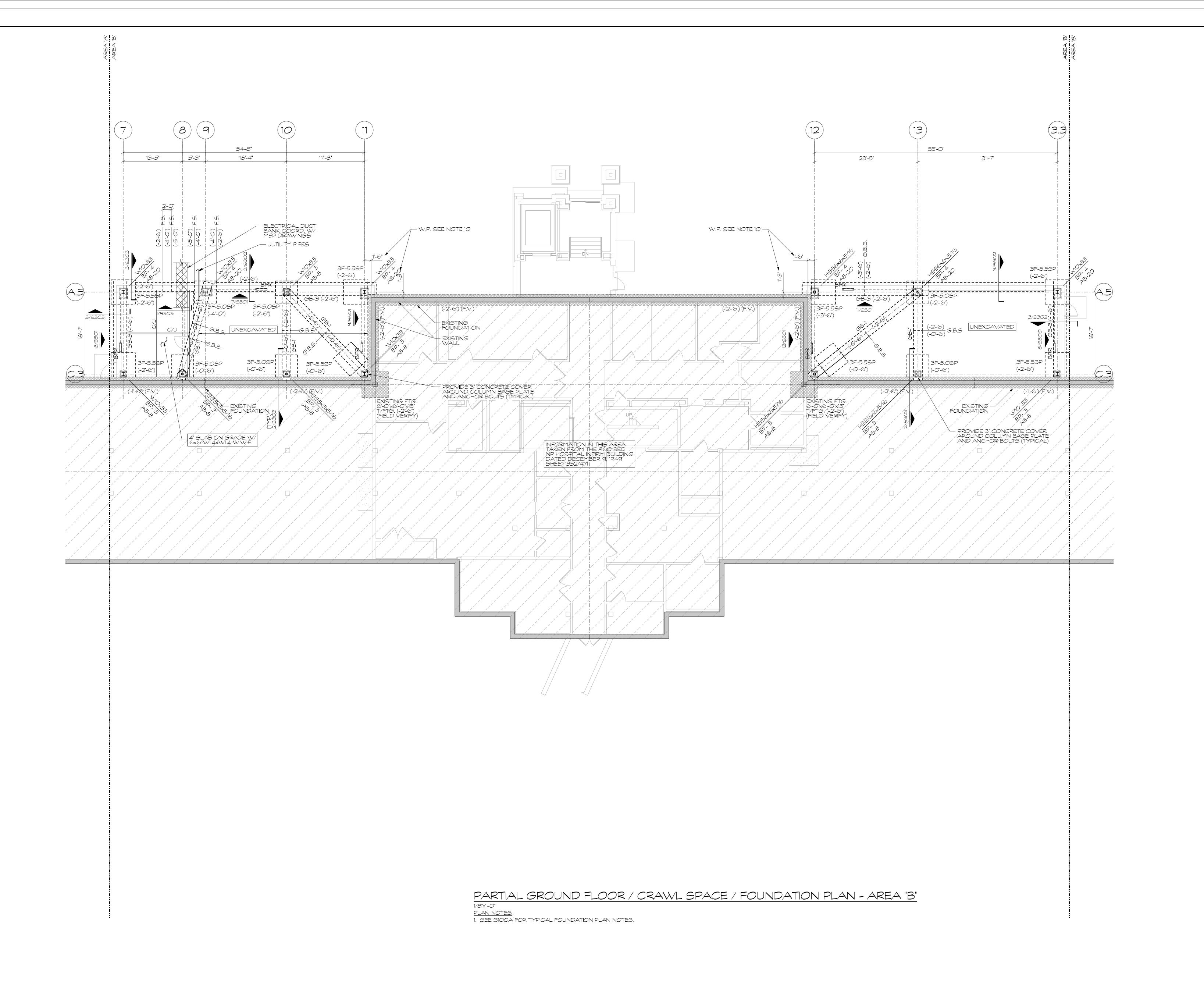
CRAWL SPACE /

FOUNDATION PLAN 
AREA "A"

Date **August 28, 2014** 

Sheet Number

**S100A** 









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Sheet Title **PARTIAL GROUND FLOOR / CRAWL SPACE / FOUNDATION PLAN -**AREA "B"

August 28, 2014

Sheet Number

**S100B** 

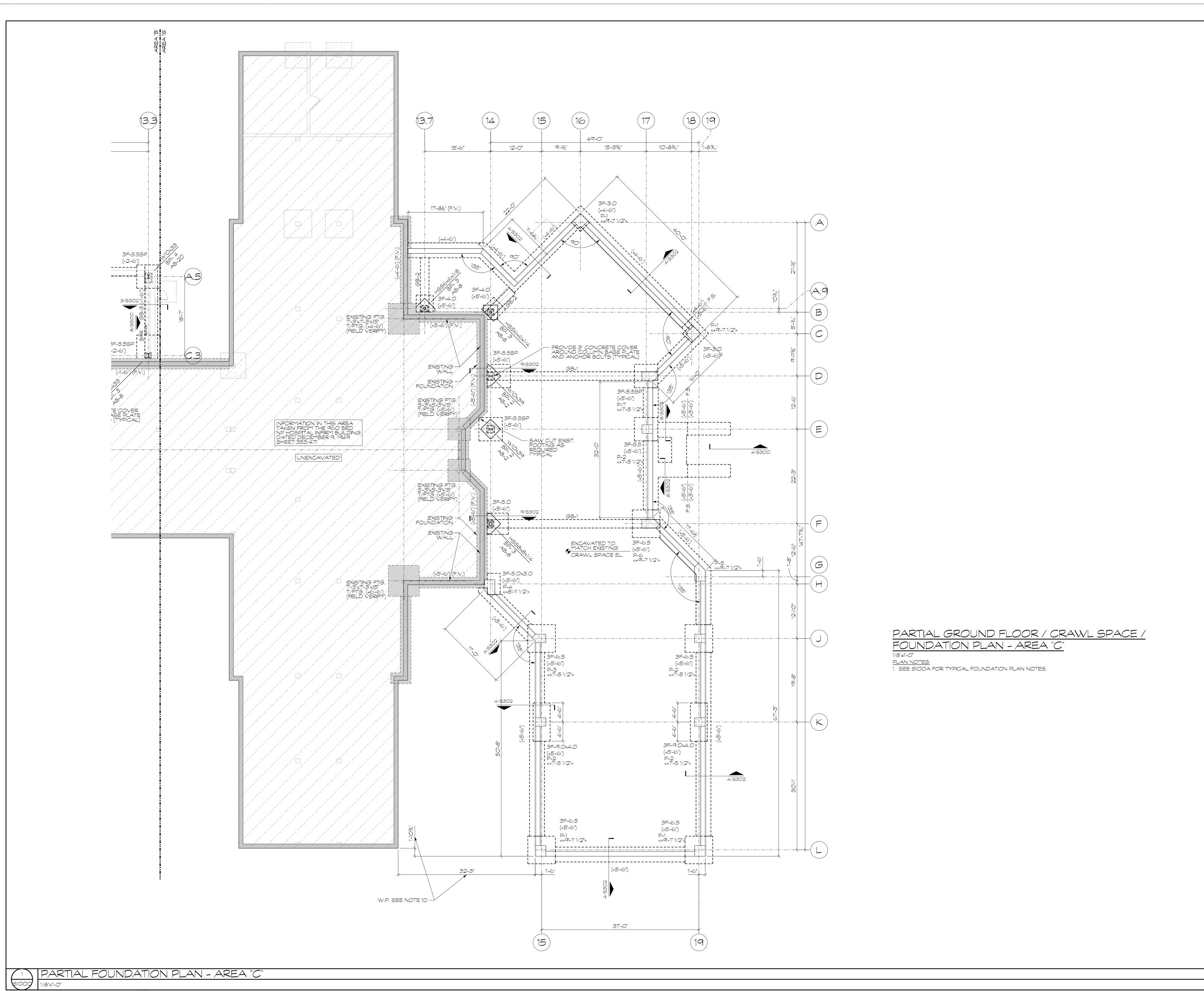
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Sheet Title **PARTIAL GROUND FLOOR / CRAWL SPACE / FOUNDATION PLAN -**AREA "C"

Sheet Number

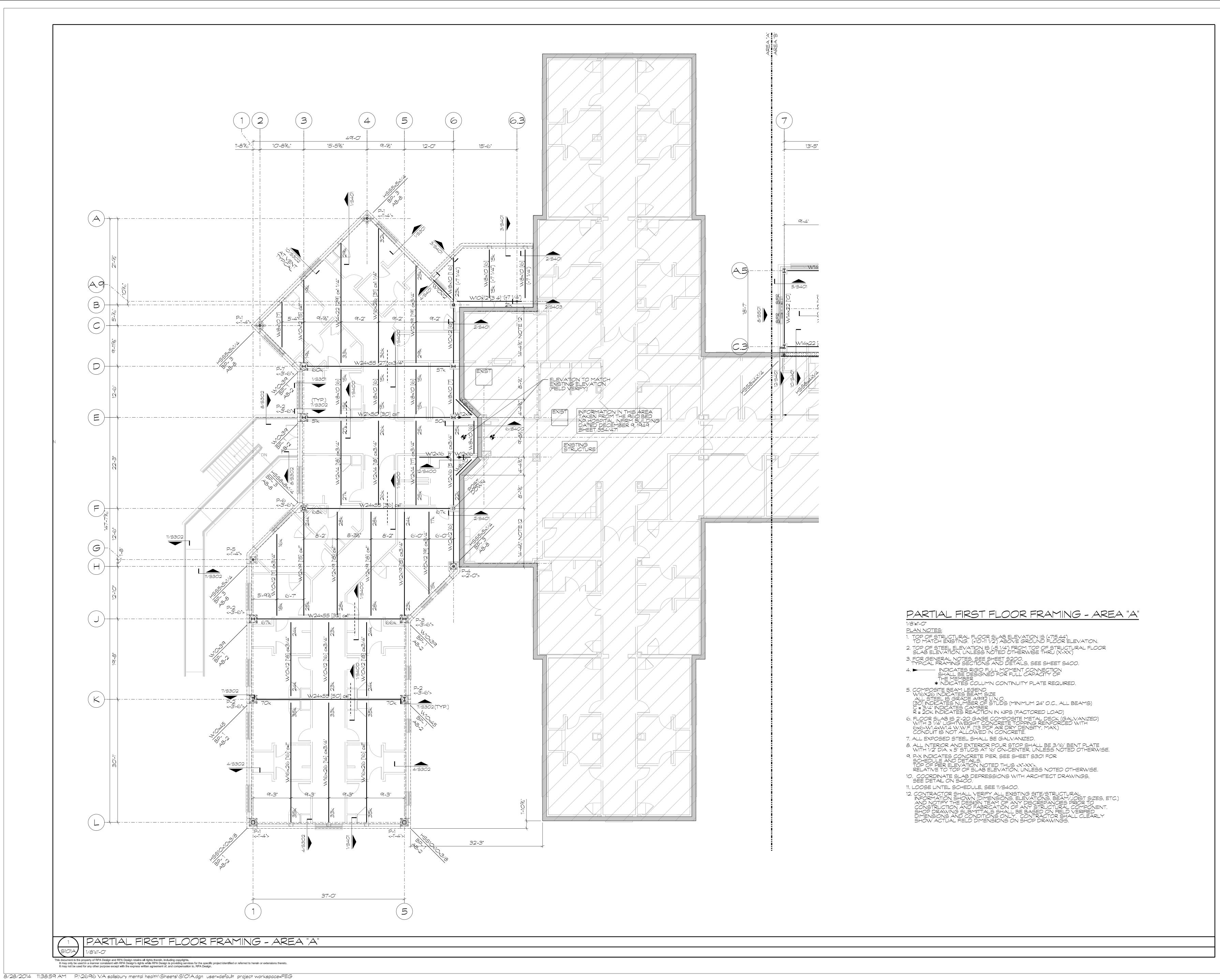
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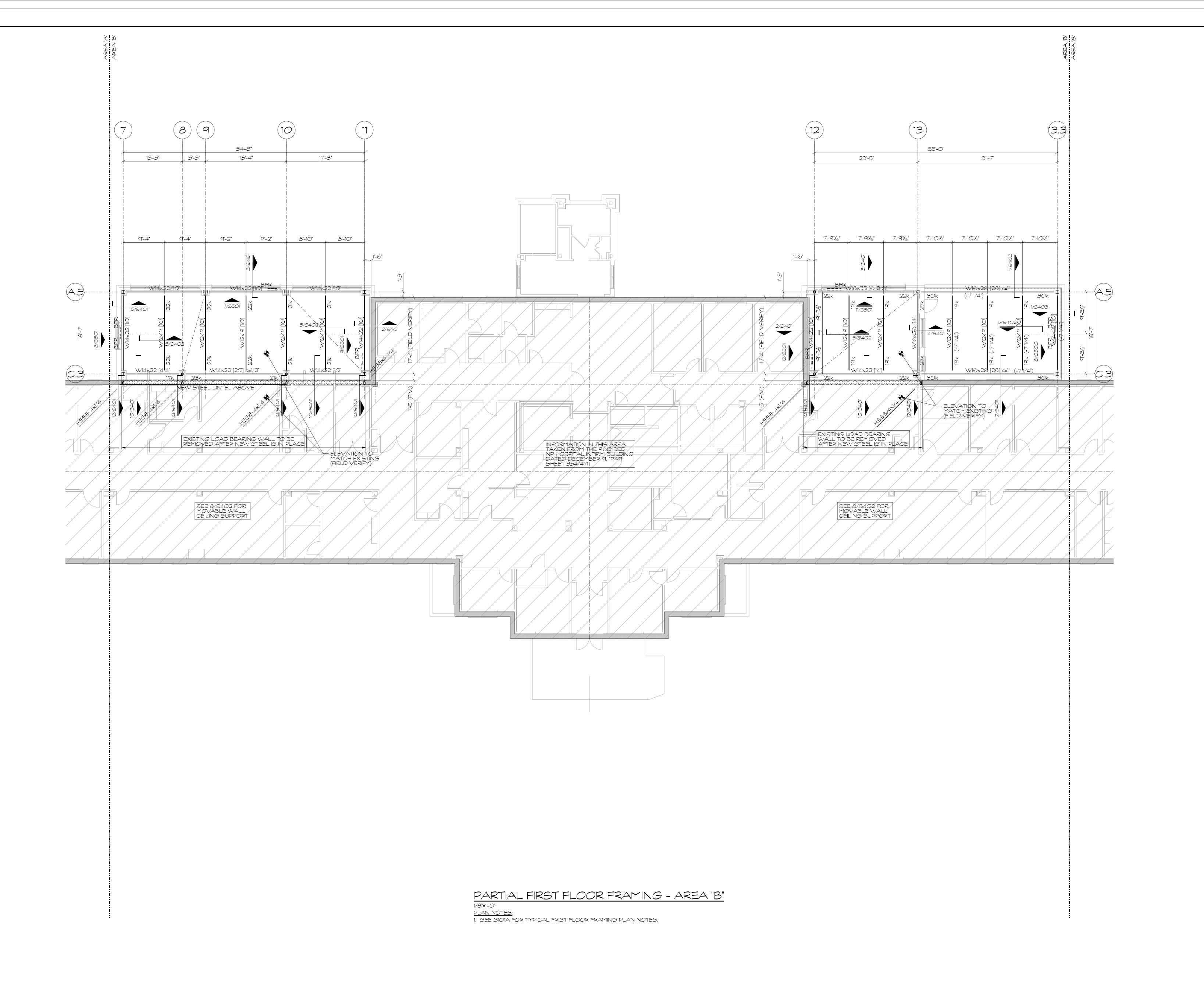
Sheet Title
PARTIAL FIRST
FLOOR FRAMING
- AREA "A"

\_\_\_\_\_ Date

August 28, 2014

Sheet Number

**S101A** 









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Sheet Title PARTIAL FIRST FLOOR FRAMING - AREA "B"

August 28, 2014

Sheet Number

**S101B** 

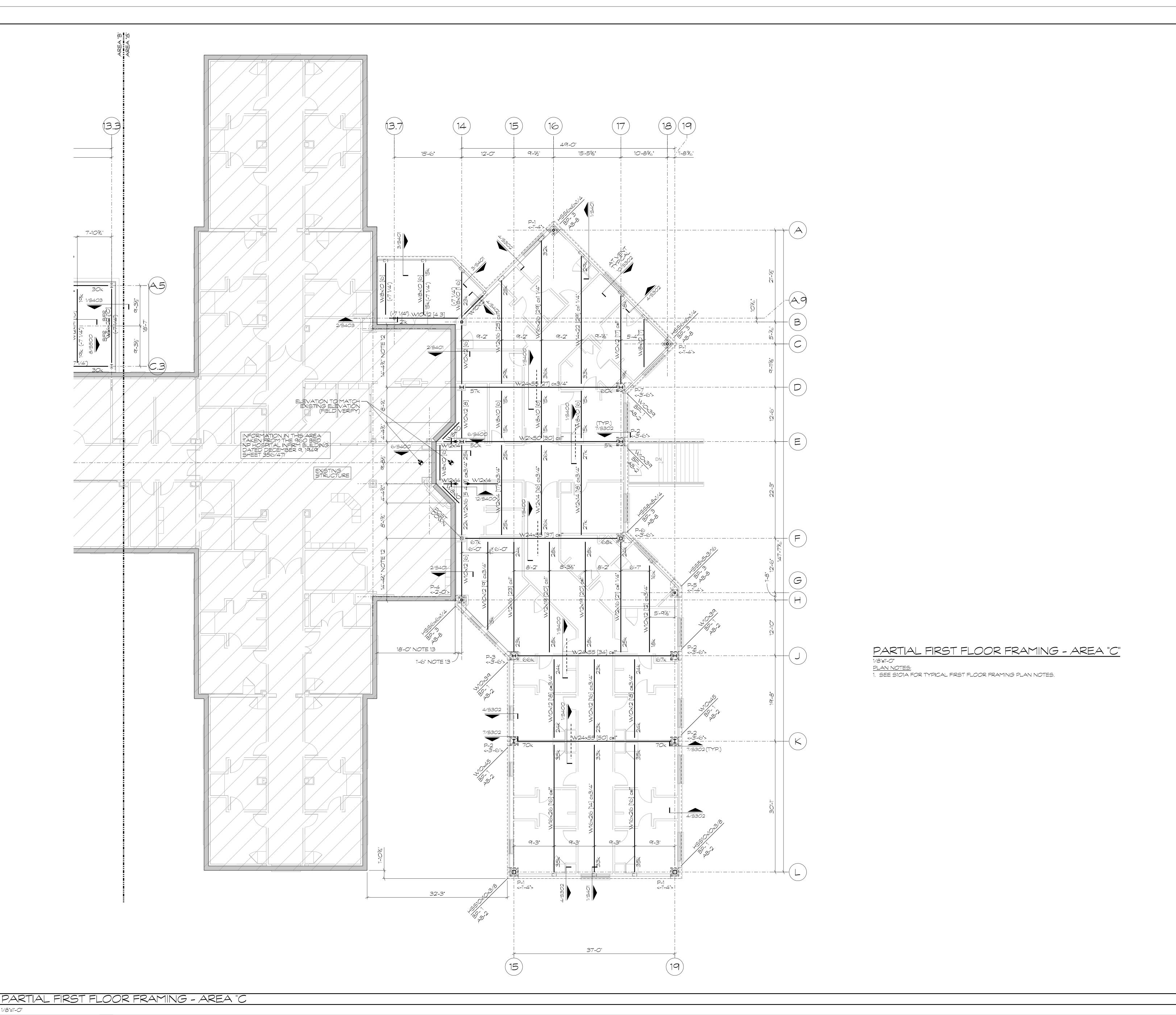
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PARTIAL FIRST FLOOR FRAMING - AREA "B"



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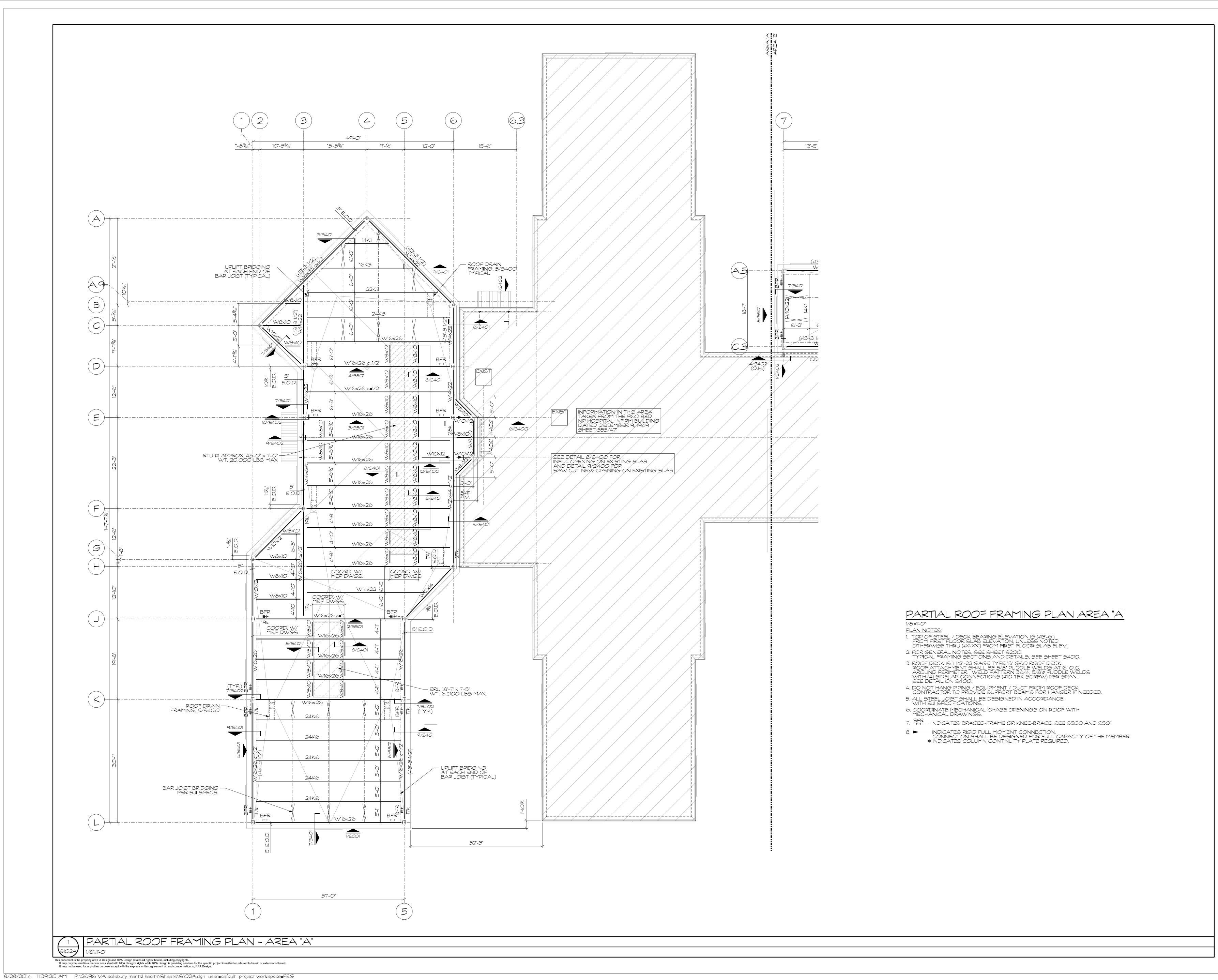


Sheet Title PARTIAL FIRST FLOOR FRAMING - AREA "C"

August 28, 2014

Sheet Number S101C

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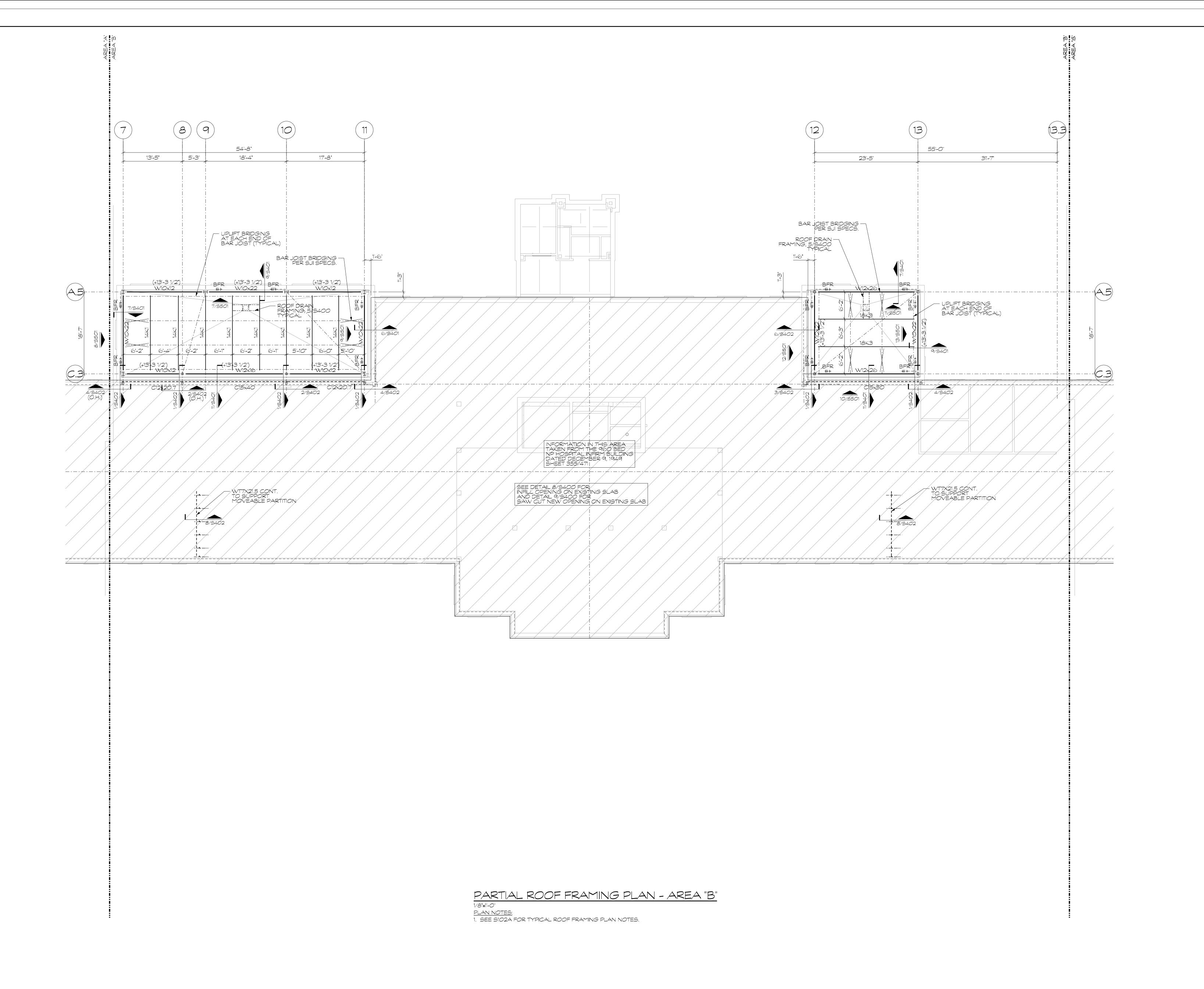
PARTIAL
ROOF FRAMING
PLAN - AREA "A"

Date

August 28, 2014

Sheet Number

**S102A** 







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Sheet Title **PARTIAL ROOF FRAMING** PLAN - AREA "B"

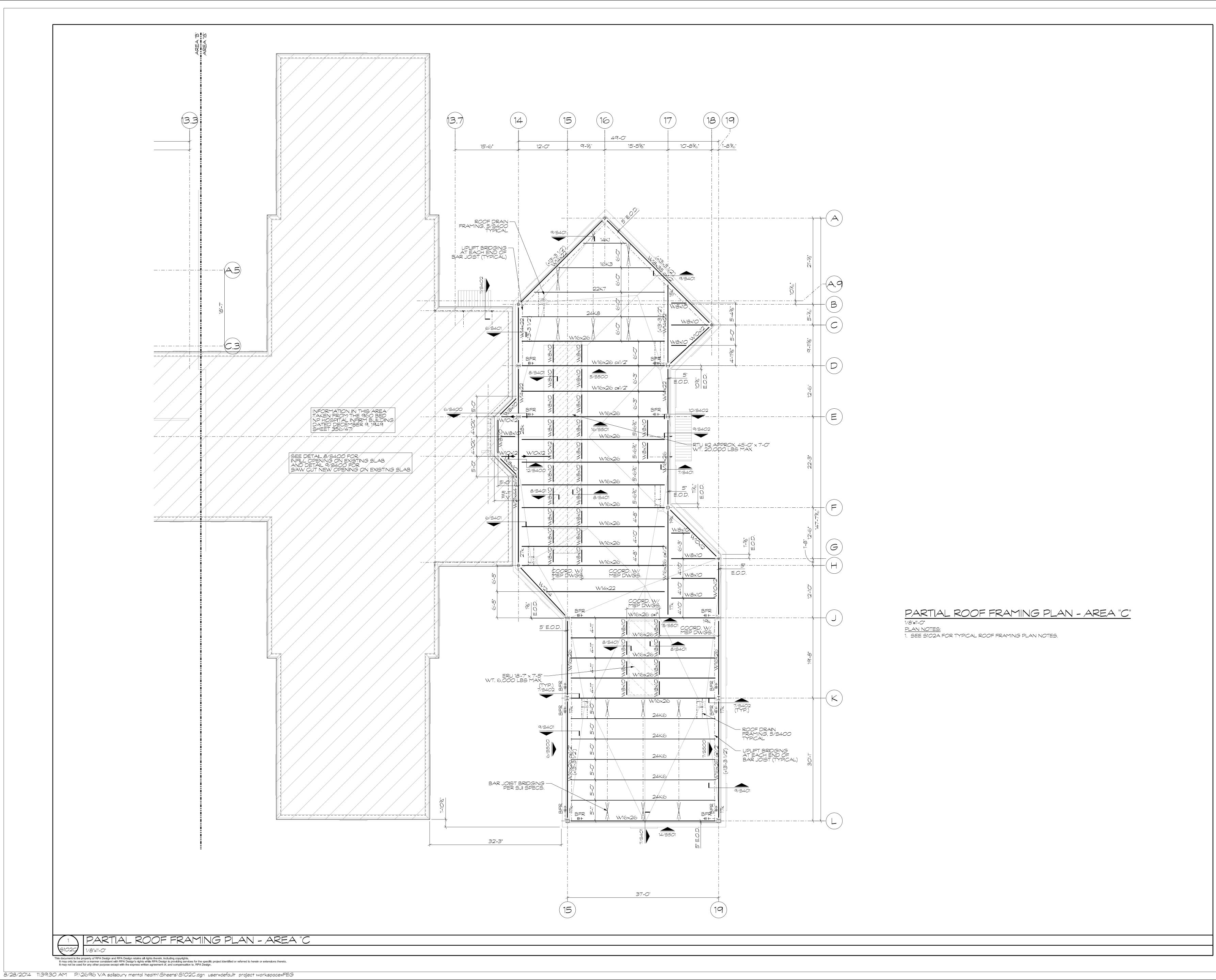
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**S102B** 

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PARTIAL ROOF FRAMING PLAN - AREA "B"



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Sheet Title
PARTIAL
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PLAN - AREA "C"

August 28, 2014

Sheet Number

**S102C** 

#### 1. GENERAL

- A. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS.
- B. THE CONTRACTOR WILL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND SHALL AT ALL TIMES TAKE ALL REASONABLE PRECAUTIONS FOR THE SAFETY OF ITS EMPLOYEES ON THE PROJECT, AND SHALL COMPLY WITH ALL APPLICABLE PROVISIONS OF FEDERAL, STATE, AND MUNICIPAL SAFETY LAWS AND BUILDING CONSTRUCTION CODES.
- C. FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL STUDY PREPARED BY ECS CAROLINA, LLP. ECS PROJECT NUMBER 08-9011, DTAED JUNE 24, 2013. FOOTINGS ARE DESIGNED FOR 3000 PSF ALLOWABLE SOIL BEARING CAPACITY, IN ACCORDANCE WITH THE SITE PREPARATION RECOMMENDATIONS INDICATED IN THE REPORT.
- D. IF EXISTING CONDITIONS MAKE IT NECESSARY TO REVISE STRUCTURAL DETAILS, ADVISE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY CHANGE.
- STEEL STUD DESIGN IS NOT PART OF THE SCOPE OF WORK INCLUDED IN THESE DOCUMENTS. STEEL STUDS INDICATED ON THESE DRAWINGS ARE FOR DESIGN INTENT ONLY. ALL COLD FORMED STEEL FRAMING IS TO BE DESIGNED BY A REGISTERED ENGINEER, TYPICALLY ENGAGED BY THE STEEL STUD FRAMING CONTRACTOR.
- THEY SHALL BE DESIGNED FOR LOADS AND DEFLECTION CRITERIA CONSISTENT WITH THE LOAD DATA ON THESE DRAWINGS AND INDUSTRY STANDARDS. COLD FORMED FRAMING ERECTION DRAWINGS SHALL CLEARLY SHOW LAYOUT, SPACINGS, SIZES, THICKNESSES, FASTENINGS, ANCHORAGES AND CONNECTION DETAILS AT A MINIMUM.
- F. STEEL STAIRS SHALL BE ENGINEERED BY A PROFESSIONAL REGISTERED ENGINEER UNLESS OTHERWISE NOTED. SHOP DRAWINGS INCLUDING PLANS, SECTIONS AND ATTACHMENTS TO STRUCTURE SHALL BE PROVIDED FOR REVIEW.

#### DESIGN CRITERIA

- A. BUILDING DESIGNED IN ACCORDANCE WITH THE 2012 NORTH CAROLINA STATE BUILDING CODE (2009 INTERNATIONAL BUILDING CODE) STEEL DESIGN AISC 13TH EDITION CONCRETE DESIGN ACI 318-08
- B. FLOOR LOADS: FIRST FLOOR 100 PSF (REDUCIBLE)

## C. ROOF LIVE LOAD = 20PSF

THE TYPICAL FLOOR LIVE LOAD ALLOWANCE OF 80 PSF HAS BEEN UTILIZED ALONG WITH LIVE LOAD REDUCTION FACTORS ALLOWED BY THE BUILDING CODE. FOR TRIBUTARY AREAS EXCEEDING 150 SQUARE FEET THE NET LIVE LOAD CAPACITY MAY BE LESS THAN 80 PSF. A STRUCTURAL EVALUATION SHOULD BE PERFORMED PRIOR TO LOCATING HEAVY FILE ROOMS, LIBRARIES ETC.

NOTE : REGARDING DEFLECTIONS AND INTERIOR NON-LOAD BEARING STEEL STUDS STEEL STUD FRAMING FROM SLAB ON GRADE TO STEEL FLOOR OR ROOF SHALL BE DESIGNED TO ACCOMMODATE VERTICAL DEFLECTION OF L/360 OF BEAM SPAN WITHOUT LOADING THE STUDS. STUDS SPANNING BETWEEN FRAMED FLOORS SHALL BE DESIGNED FOR VERTICAL MOVEMENT OF L/540 OF BEAM SPAN.

### D. ROOF SNOW LOAD

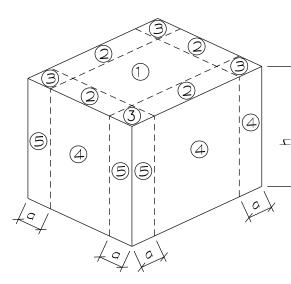
- Pg = 15.0 PSF Pf = 18.0 PSF Ce = 1.0 Ct = 1.0 GROUND SNOW LOAD 2. FLAT-ROOF SNOW LOAD
- 2. FLAT-RUOF SINOW LUAD
  3. SNOW EXPOSURE FACTOR
  4. THERMAL FACTOR
  5. SNOW LOAD IMPORTANCE FACTOR Is = 1.2
- E. WIND LOADS DESIGNED IN ACCORDANCE WITH THE 2009 INTERNATIONAL BUILDING CODE (WITH 2012 NORTH CAROLINA STATE AMENDMENTS), AND ANSI/ASCE 7-05

  - 1. BASIC WIND SPEED (3-SECOND GUST) = 90 MPH
    2. WIND IMPORTANCE FACTOR IW = 1.15
    3. WIND EXPOSURE B
    4. WIND BASE SHEAR:
    EAST WING: VX = 50 k VY = 20 k
    WEST WING: VX = 50 k VY = 20 k
    DINNING: VX = 6 k VY = 20 k

  - DINNING: VX = 6 k VY = 20 k CONFERENCE: VX = 7 k VY = 11 k
- 5. INTERNAL PRESSURE COEFFICIENT GCpi = .18±
- . COMPONENTS AND CLADDING SHALL BE DESIGNED FOR THE WIND PRESSURE TABULATED BELOW. DEFLECTION CALCULATIONS MAY USE 70% OF TABULATED

ZONE PER FIG. 6-3	EFFECTIVE WIND AREA	POSITIVE PRESSURE PSF	NEGATIVE PRESSURE PSF
1	50	+10.0	-17.0
2	50	+10.0	-22.0
3	50	+10.0	-27.0
4	10	+16.0	-18.0
4	20	+15.5	-17.0
4	50	+14.5	-16.0
4	100	+14.0	-15.0
5	10	+16.0	-22.0
5	20	+15.5	-20.0
5	50	+14.5	-18.0
5	100	+14.0	-17.0

7. SEE ROOF DIAPHRAGM BELOW FOR ATTACHMENT INFORMATION.



ATTACHMENT SEE PLAN

# F. EARTHQUAKE DESIGN DATA

- 1. OCCUPANCY CATEGORY: III 2. SPECTRAL RESPONSE COEFFICIENTS: SDS = 0.209 SDI = 0.108
- 3. SITE CLASS: C 4. BASIC SEISMIC-FORCE-RESISTING SYSTEM: STRUCTURAL STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.
- 5. SEISMIC BASE SHEAR:
  EAST WING: VEQ = 95.0k
  WEST WING: VEQ = 95.0k
  DINNING: VEQ = 20.0k

G. MECHANICAL ANCHORAGE: IP = 1.0

- CONFRENCE: VEQ = 16.0k 6. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
- 7. R = 3, OMEGA = 3, Cd = 3
- 8. SEISMIC DESIGN CATEGORY: B 9. SEISMIC IMPORTANCE FACTOR: le = 1.25
- CONCRETE A. REINFORCING STEEL ASTM A615 GRADE 60 WELDED WIRE FABRIC ASTM A185 MESH
- B. UNLESS OTHERWISE NOTED ON THE DRAWINGS, LAP SPLICES SHALL BE A CLASS B SPLICE.
- C. CONCRETE (28 DAY STRENGTH)
- FOUNDATIONS/PIERS/SLAB ON GRADE: 3000 PSI NORMAL WEIGHT CONCRETE
  EXTERIOR SLAB ON GRADE: 4500 PSI NORMAL WEIGHT CONCRETE (AIR ENTRANCE)
  ELEVATED SLAB: 3000 PSI LIGHT WEIGHT CONCRETE (113 PCF MAX)

### 4. STRUCTURAL STEEL (COMPOSITE)

- A. ALL WIDE FLANGE STRUCTURAL STEEL SHALL BE ASTM A992.
  ALL OTHER STEEL SHALL BE ASTM A36, UNLESS NOTED OTHERWISE.
  TUBE STEEL SHALL CONFORM TO ASTM A500, GRADE B (Fy = 46 KSI)
  PIPE STEEL SHALL BE ASTM A53 GRADE B, OR ASTM 501 (Fy = 36 KSI)
  ALL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC LATEST
  EDITION. ALL BOLTED CONNECTIONS ARE TO BE WITH A-325 HIGH STRENGTH BOLTS.
  CONNECTIONS ARE TO DEVELOP THE REACTIONS SHOWN ON THE DRAWINGS OR ONE HALF OF
  THE TOTAL LOAD CAPACITY SHOWN IN THE MAXIMUM TOTAL UNIFORM LOAD TABLES OF THE AISC
  MANUAL OR 10 KIPS FROM LRFD, WHICHEVER IS GREATER. FIELD WELDS SHALL ONLY BE MADE BY
  OPERATORS CERTIFIED BY TESTS DESCRIBED IN AWS D1.1, SEE SPECIFICATIONS.
- B. FABRICATORS SHOP DRAWING SHALL SHOW AND NOTE ALL MATERIAL REQUIRED WITH RELATIVE LOCATIONS AND SUFFICIENT DETAILS FOR PROPER FABRICATION AND ERECTION IN ACCORDANCE WITH ALL CONTRACT DRAWINGS AND DOCUMENTS.
- . STEEL DECK SIZE AND GAGE SHALL BE AS INDICATED ON PLANS. ALL DECK SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATION, THREE SPAN MINIMUM. NO EQUIPMENT SHALL HANG FROM STEEL DECK. RESPECTIVE CONTRACTOR TO PROVIDE SUPPORTE AS REQUIRED. CEILING, LIGHTS, AND DUCT MAY BE HUNG FROM DECK WITH
- D. ROOF AND FLOOR OPENINGS NOT SHOWN ON PLANS SHALL BE FRAMED WITH 3 1/2"X3 1/2"X1/4" ANGLES.
- E. JOIST SHALL BE DESIGNED FOR A NET UPLIFT PRESSURE OF 12 PSF DUE TO WIND. PROVIDE A SINGLE LINE OF BOTTOM CHORD BRIDGING AT THE FIRST BOTTOM CHORD PANEL POINT AS RECOMMENDED BY SJI.
- F. SHEAR STUDS ARE TO BE 3/4" DIAMETER x 4" LONG AND NOT EXTEND LESS THAN 1 1/2" ABOVE TOP OF STEEL DECK AND NOT HAVE LESS THAN 1/2" CONCRETE COVER. FIELD APPLIED TO BEAMS THROUGH DECK. SPACE UNIFORMLY EACH SIDE OF MIDSPAN OF BEAM EXCEPT AS NOTED. BEAMS NOT MARKED AS COMPOSITE TO HAVE STUDS 24" ON CENTER. SUGGESTED FIELD PROCEDURE FOR WELDING STUDS THROUGH DECK AS FOLLOWS:
  - 1. DECK SHALL BE FREE OF ALL FOREIGN MATERIALS AND MOISTURE.
  - SET UP WELDING GUN PRIOR TO EACH WORKDAY AND AFTER ANY LENGTHY WORK STOPPAGE. GUN OPERATOR IS TO WELD TWO TEST STUDS, INSPECT AND BEND STUDS 45 DEGREES. REPEAT THIS PROCEDURE AS REQUIRED UNTIL PROPER WELD RESULTS ARE OBTAINED. 3. PROCEED WITH STUD WELDING AFTER SATISFYING ABOVE REQUIREMENTS.
  - 4. VISUALLY INSPECT EACH STUD FOR PROPER WELD. FILLET WELD ALL AROUND THOSE STUDS NOT HAVING PROPER GUN WELD, ALL STUDS REQUIRING FILLET WELD, IN ADDITION TO GUN WELD, ARE TO BE FIELD TESTED BY BENDING 15 DEGREES. 5. STUDS SHALL BE FIELD TESTED BY INDEPENDENT TEST LAB IN ACCORDANCE WITH SPECIFICATIONS.
  - 6. STUDS THAT FAIL SHALL BE REPLACED.

## G. ► INDICATES MOMENT CONNECTION.

H. CONNECTIONS NOT DETAILED IN THE CONSTRUCTION DOCUMENTS SHALL BE DESIGNED BY FABRICATOR OR DETAILER. ALL CONNECTIONS SHALL BE SIGNED AND SEALED APPROVED BY A REGISTERED PROFESSIONAL ENGINEER.

## 5. MASONRY

- A. MASONRY MORTAR TO BE TYPE "S". PROVIDE STANDARD HORIZONTAL JOINT REINFORCING AT 16" O.C. AT ALL BLOCK WALLS. PROVIDE 1#4 BAR WITH CELL GROUTED SOLID AT ENDS, INTERSECTIONS, AND OPENINGS OF CMU WALLS.
- B. GROUT FOR HOLLOW UNIT SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND SHALL CONFORM TO ASTM 0476.
- C. QUALITY ASSURANCE LEVEL B ACI 530-08 (1.18.2)
- 1. VERIFY COMPLIANCE WITH APPROVED SUBMITTALS.
- AS MASONRY CONSTRUCTION BEGINS, VERIFY THE FOLLOWING ARE IN COMPLIANCE
   - PROPORTIONS OF SITE PREPARED MORTAR
   - CONSTRUCTION OF MORTAR JOINTS
   - LOCATION OF REINFORCEMENT AND CONNECTORS
- 3. PRIOR TO GROUTING, VERIFY THE FOLLOWING ARE IN COMPLIANCE - GROUT SPACE
  - PLACEMENT OF REINFORCEMENT AND CONNECTORS
  - PROPORTIONS OF SITE-PREPARED GROUT
  - CONSTRUCTION OF MORTAR JOINTS
- 4. DURING CONSTRUCTION, VERIFY THE FOLLOWING IS IN COMPLIANCE:
   SIZE AND LOCATION OF STRUCTURAL ELEMENTS
   SIZE, TYPE AND LOCATION OF ANCHORS
   INCLUDING ANCHORAGE TO STRUCTURAL MEMBERS OR FRAMES
   PROTECTION, PREPARATION DURING COLD WEATHER
   GROUT PLACEMENT
- 5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS
- 6. MINIMUM TESTS FOR LEVEL B ARE VERIFICATION OF SLUMP - VERIFICATION OF f'm PRIOR TO CONSTRUCTION

- A. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL.
- B. THE GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO ENGINEER FOR REVIEW OF THE FOLLOWING APPLICABLE ITEMS:
- 1. ALL STRUCTURAL STEEL INCLUDING MISCELLANEOUS EMBEDMENTS. STEEL FABRICATION SHOP DRAWINGS ARE TO BE SUBMITTED TO STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION. THE STRUCTURAL ENGINEER IS TO REVIEW DRAWINGS FOR COMPLIANCE WITH DESIGN DOCUMENTS. ENGINEER IS TO REVIEW TYPE OF MATERIAL, MEMBER SIZES, AND CONNECTIONS AND RETURN DRAWINGS TO THE STEEL FABRICATOR NOTING ANY DISCREPANCIES. THE STEEL FABRICATOR IS RESPONSIBLE FOR ALL FABRICATION DIMENSIONS.
- 2. OPEN WEB STEEL JOISTS (\*).
- 3. REINFORCING STEEL.
- 4. STRUCTURAL STEEL DECK. 5. CONCRETE MIX DESIGNS.
- 6. LIGHT GAGE STEEL STUD, JAMBS, AND HEADERS AND/OR TRUSSES WITH CALCULATIONS (\*) 7. ANCHOR BOLT SUBMITTAL
- 8. MECHANICAL EQUIPMENT REQUIRING STRUCTURAL SUPPORT
- 9. CONCRETE CURING MATERIALS
- 10. MECHANICAL ANCHOR SUBSTITUTIONS

ITEMS MARKED (\*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT LOCATION.

- A. ADHESIVE ANCHORAGES HAVE BEEN SPECIFIED IN ACCORDANCE WITH HILTI DESIGN DATA.
  THE CONTRACTOR MAY SUBSTITUTE ANY OF THE BELOW LISTED MANUFACTURERS PROVIDED
  SIGNED AND SEALED CALCULATIONS ARE SUBMITTED FOR ENGINEER REVIEW.
  THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTE PRODUCT MEETS
  OR EXCEEDS THE STRENGTH OF THE BASIS OF DESIGN ANCHORAGE.
- B. ANCHORAGE TO CONCRETE

  1. ADHESIVE EPOXY ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI355.4 AND ICC-ES AC308 AND MEET LOAD SPECIFIED SUCH AS:
  THE FOLLOWING OR EQUAL:
  a. HILTI HIT-RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM (ICC ESR-2322)
  b. HILTI HIT-HY MAX-SD ADHESIVE (ICC ESR-3013)
  c. POWERS PE-1000+ (ICC ES ESR 2583)
  d. SIMPSON STRONG TIE "SET-XP" (ICC-ES ESR-2508)
  e. OR EQUAL ADHERING TO ICC ES AC 308

  - FOR HILTI PRODUCTS, STEEL ELEMENTS SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD, HILTI HIS-N INTERNALLY THREADED INSERTS OR CONTINUOUSLY DEFORMED STEEL REBAR. FOR POWERS PRODUCTS, STEEL ELEMENTS SHALL BE ASTM A307-GRADE C, ASTM A 193 GRADE B7 TYPE 304 OR 316 STAINLESS STEEL (F 593, CONDITION CW) OR CONTINUOUSLY DEFORMED STEEL REBAR. FOR EQUAL PRODUCTS USE ACCORDING TO MANUFACTURER'S SPECIFICATIONS THAT MEET OR EXCEED LOADING REQUIREMENTS.
  - 2. MECHANICAL EXPANSION ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC 193 AND MEET LOAD SPECIFIED SUCH AS a. HILTI KWIK BOLT TZ ANCHORS (ICC ES ESR-1917) b. HILTI HSL-3 HEAVY DUTY SLEEVE ANCHORS (ICC ES ESR 1545) c. POWERS POWER-STUD SD-1 (ICC ES ESR 2818) d. POWERS POWER-BOLTS+ d. SIMPSON STRONG-TIE "TITEN-HD"(ICC-ES ESR 2713) e. SIMPSON STRONG TIE "STRONG BOLT 2" (ICC-ES ESR 3037) f. OR EQUAL ADHERING TO ICC ES AC 193.

  - 3. MECHANICAL UNDERCUT ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC ES AC 193 AND MEET LOAD SPECIFIED SUCH AS a. HILTI HDA UNDERCUT ANCHORS b. POWERS ATOMIC+ UNDERCUT ANCHORS (ICC ES ESR 3067 c. OR EQUAL ADHERING TO ICC ES AC 193
  - 4. CONCRETE SCREW ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC ES AC 193 AND MEET LOAD SPECIFIED SUCH AS

    a. HILTI KWIK HUS-EZ CONCRETE SCREW ANCHORS (ICC ES ESR 3027),

    b. POWERS WEDGE BOLT+ (ICC ES ESR 2526)

    c. OR EQUAL ADHERING TO ICC ES AC 193
- C. ANCHORAGE TO SOLID GROUTED CONCRETE MASONRY

  1. ADHESIVE (EPOXY) ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC ES AC58 AND MEET LOAD SPECIFIED SUCH AS

  a. HILTI HIT-HY 150 MAX ADHESIVE ANCHORING SYSTEM (ICC ES ESR-1967)

  USING STEEL ELEMENTS HILTI HAS-E CONTINUOUSLY

  THREADED ROD OR CONTINUOUSLY DEFORMED STEEL REBAR

  b. SIMPSON STRONG TIE "SET" (ICC ES ESR-1772)

  c. OR EQUAL ADHERING TO ICC ES AC 58

- 2. MECHANICAL .ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ISS ES ACOI OR ISS ES ACIOG AND MEET LOAD SPECIFIED SUCH AS a. HILTI KWIK BOLT-3 EXPANSION ANCHORS, (ISS ES ESR-1385) b. POWER STUD SD-1 (ICC ES ESR -2966) c. SIMPSON STRONG TIE "TITEN HD" (ICC ES ESR-1056) d. SIMPSON STRONG TIE "WEDGE ALL" (ICC ES ESR 1396) c OR EQUAL ADHERING TO ICC ES ESR AC 106.
- D. ANCHORAGE TO HOLLOW/MULTI-WYTHE MASONRY AND/OR CONCRETE MASONRY

  1. ADHESIVE (EPOXY) ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC ES AC 58 AND MEET LOAD SPECIFIED SUCH AS

  a. HILTI HIT-HY-20 ADHESIVE ANCHORING SYSTEM (ICC ES ESR4815)

  USING STEEL ELEMENTS HILTI HAS-E CONTINUOUSLY

  THREADED ROD OR CONTINUOUSLY DEFORMED STEEL REBAR

  b. POWERS T308+ ADHESIVE ANCHOR (ICC ES ESR-3149) BRICK MASONRY ONLY

  c. SIMPSON STRONG TIE "SET" (ICC ES ESR 1772) CONCRETE MASONRY ONLY

  c. OR EQUAL ADHERING TO ICC ES 58
- 2. MECHANICAL ANCHORS SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC ES AC 106 AND MEET LOAD SPECIFIED SUCH AS a. SIMPSON STRONG TIE "TITEN HD" CONCRETE MASONRY ONLY b. OR EQUAL ADHERING TO ICC ES AC 106
- E. INSTALLATION
  1. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING ALL FASTENERS ACCORDING TO MANUFACTURER'S SPECIFICATIONS AND AS REQUIRED BY CODE. A PRE-CONSTRUCTION MEETING AND OR TRAINING IS RECOMMENDED TO ENSURE PROPER INSTALLATIONS AT NO COST TO THE OWNER.
  2. IF EXISTING CONDITIONS PREVENT INSTALLATION AS SHOWN ON DRAWINGS OR AS RECOMMENDED, CONTRACTOR SHALL CONTACT ENGINEER OR ANCHOR REPRESENTATIVE PRIOR TO INSTALLATION OF ANCHORS FOR ALTERNATE RECOMMENDATION.
  3. CONTRACTOR SHALL BE SURE ALL SURFACES ARE CLEAN AND ARE PREPARED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.
  4. ALL ANCHORS INSTALLATIONS SHALL BE PLUMB, NEAT AND LEVEL UNLESS OTHERWISE NOTED.
  5. ARCHITECT/ENGINEER MAY REQUIRE PULLOUT OR SHEAR TESTS TO DETERMINE ADEQUACY OF ANCHORS AT NO COST TO THE OWNER.

Veterans Affairs

Department of

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**VAMC SALISBURY BUILDING 11-ADDITIONS** & RENOVATION

City, State Salisbury, NC

VA Contract Number VA246-12-C-0089

Issued For

Item Date

Description

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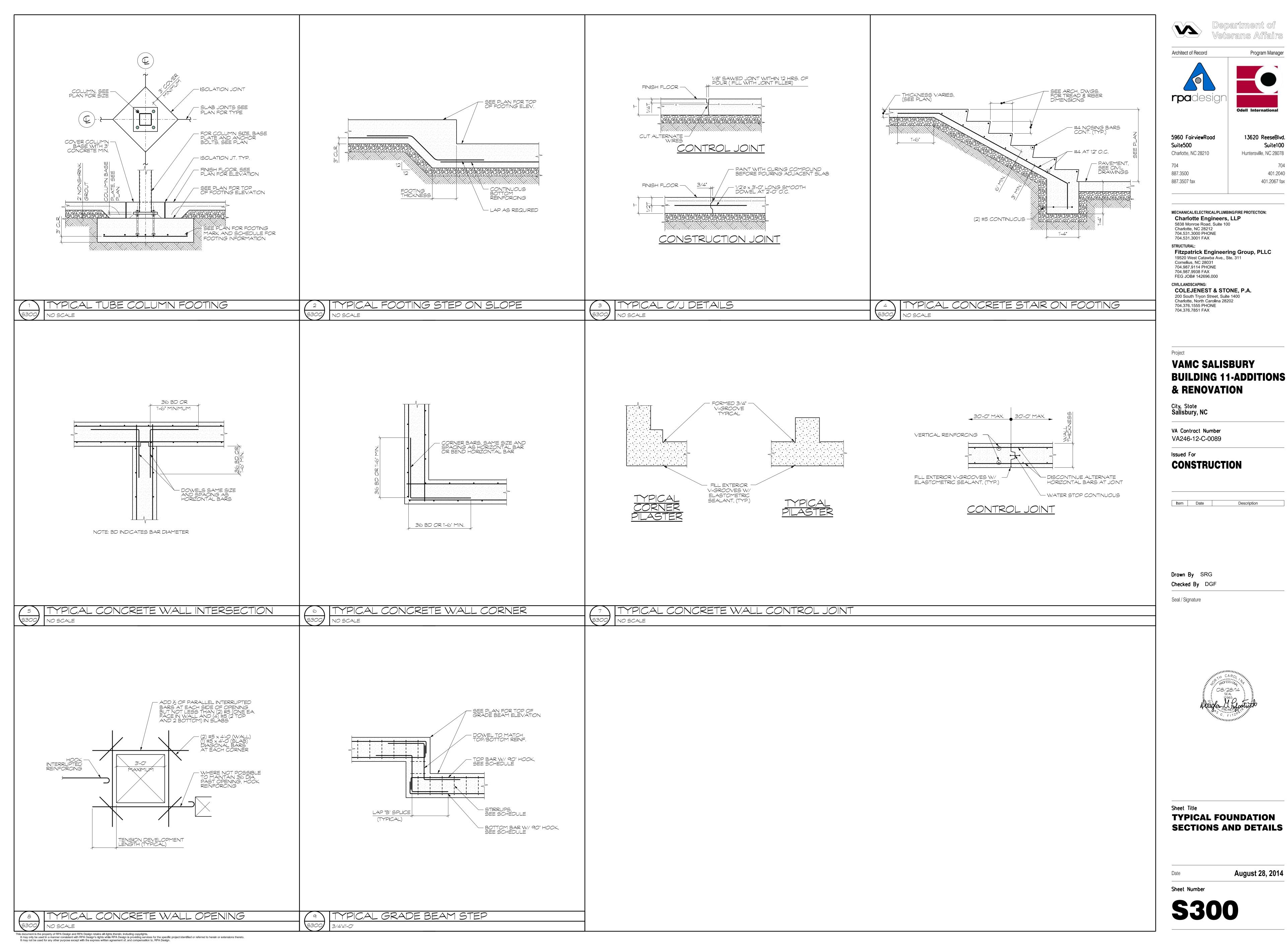
Sheet Title **GENERAL NOTES** 

August 28, 2014

Sheet Number

GENERAL NOTES

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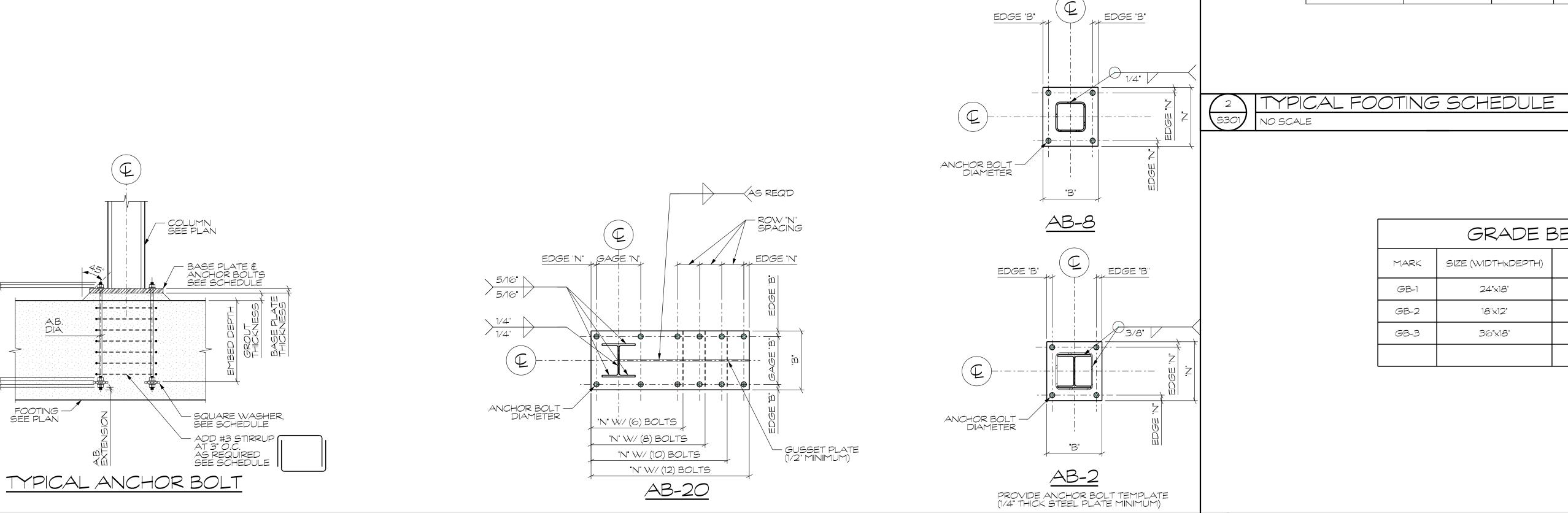


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**BUILDING 11-ADDITIONS** 

						COLL	JMN BA	SE PLAT	E SCHE	DULE					
MARK	Ν	SIZE (in)	+	GAGE N	N GAGE B EDGE N	N EDGE B	ROW N	ANCHOR BOLTS	A.B. GRADE	GROUT THICKNESS	EMBED DEPTH	SQUARE WASHER	A.B. LAYOUT DETAIL	REMARKS	
(16) BP-1	18"	18"	2"	-	- 2"	2"	-	(4) 1"	A36	2"	1'-8"	P1/2" × 3"	AB-2	ADD (8) #3 STRIRRUPS	PROVIDE ANCHOR BOLT TEM (1/4" THICK STEEL PLATE MINII
(4) BP-2	18"	18"	2"	-	- 2"	2"	-	(4) 1"	A36	2"	1'-0"	P1/2" × 3"	AB-2	ADD (6) #3 STRIRRUPS	PROVIDE ANCHOR BOLT TEM (1/4" THICK STEEL PLATE MINII
(23) BP-3	14"	14"	3/4"	-	- 1-1/2"	1-1/2"	-	(4) 3/4"	A36	2"	8"	P3/8" × 3"	AB-8	-	
(8) BP-4	25"	18"	3/4"	14"	- 2"	2"	-	(6) 3/4"	A36	2"	10"	P3/8" x 3"	AB-20	-	

	SPREAD FOOTING SCHEDULE  BEARING CAPACITY 3000 psf								
MARK	SIZE	THICKNESS	BOTTOM BARS	TOP BARS	REMARKS				
(4) 3F-3.0	3'-O" SQ.	1'-0"	6#5 E.W.	-	-				
(4) 3F-4.0	4'-0" SQ.	1'-0"	6#5 E.W.	-	-				
(2) 3F-5.0	5'-0" SQ.	1'-0"	6#5 E.W.	-	-				
(2) 3F-5.0x3.0	5'-0" × 3'-0"	1'-2"	6#5L × 3#5S	-	-				
(3) 3F-5.5	5'-6" SQ.	1'-2"	6#5 E.W.	-	-				
(10) 3F-6.5	6'-6" SQ.	1'-4"	10#5 E.W.	-	-				
(4) 3F-9.0×4.0	9'-0" × 4'-0"	1'-6"	6#6L × 13#5S	-	-				
(6) 3F-5.0SP	5'-0" SQ.	1'-6"	5#6 E.W.	5#4 E.W.	-				
(13) 3F-5.5SP	5'-6" SQ.	1'-6"	5#6 E.W.	5#4 E.W.	-				



	GRADE E	BEAM SCHE	DULE

	GRADE BEAM SCHEDULE						
MARK	SIZE (WIDTH×DEPTH)	REINFORCING	STIRRUPS				
GB-1	24 <b>"</b> ×18"	(3) #8 TOP (3) #8 BOTTOM	#3 AT 9" O.C.				
GB-2	18"×12"	(2) #6 TOP (2) #6 BOTTOM	#3 AT 6" O.C.				
GB-3	36"x18"	(3) #8 TOP (3) #8 BOTTOM	#3 AT 9" O.C.				

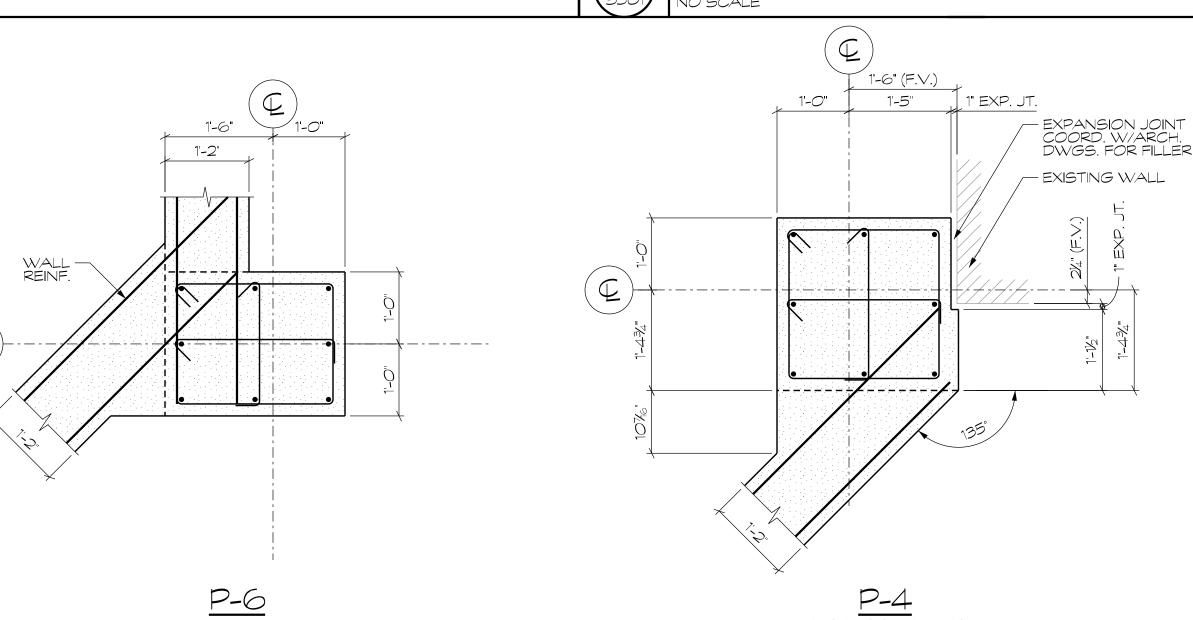
YPICAL BASE PLATE AND ANCHOR BOLT DETAILS

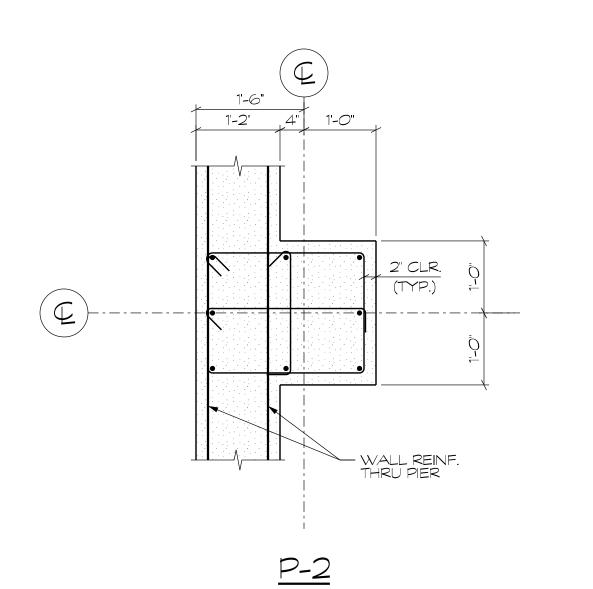
COLUMN, SEE PLAN —

PROVIDE L3x3x1/4 — FOR DECK SUPPORT AS REQUIRED

GROUT SOLID — PIER POCKET

TYPICAL GRADE BEAM SCHEDULE

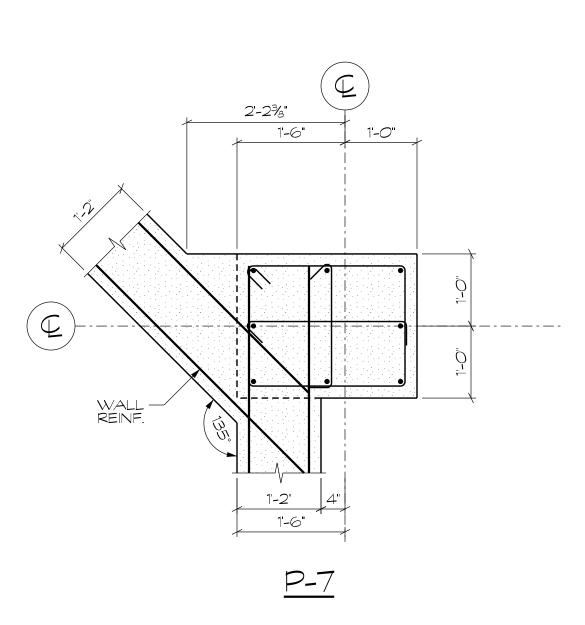


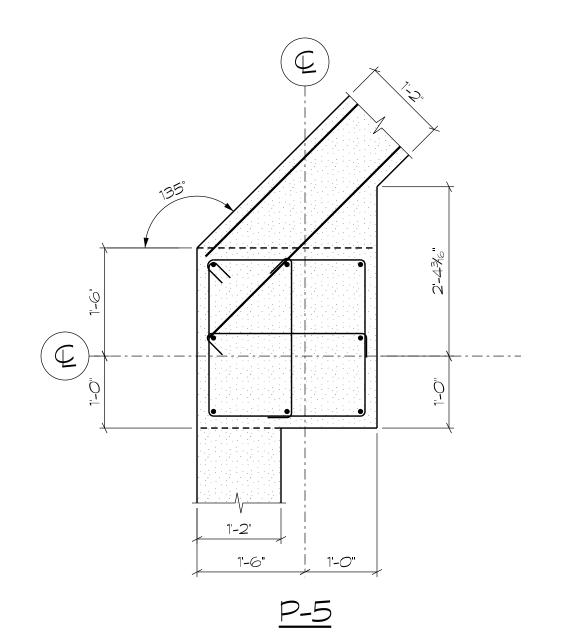


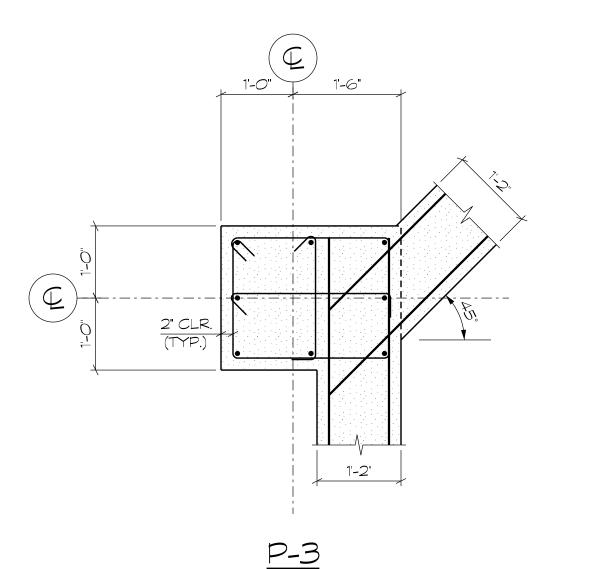
	<i>V</i>		
<i>i</i> 1 )		<u></u>	ELEVATION, SEE PLAN
RTICAL REINF.  SCHEDULE  ODEGREE STD.  OK AT TOP			
2" CLR. TO TIES		ADDITIONA AT 3" O.C. SEE SCHEL	
TYPICAL 5, SEE SCHEDULE — 2 SIZE AND		DOWELS, SEE SCHED	DULE
NG NG		TOP OF FOI SEE PLAN	UNDATION SYSTEM
TYPICAL	PIER ELE	EVATION	

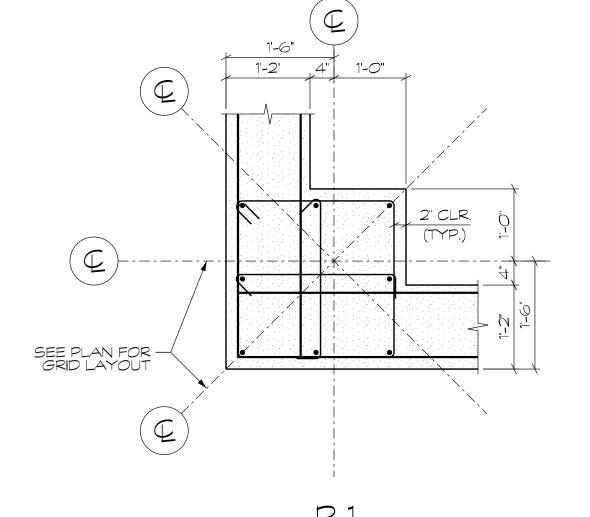
TOP OF PIER ELEVATION, SEE PLAN

	PIER SCHEDULE							
MARK	PIER SIZE	VERTICAL REINF. WITH 90° STD. HOOK AT TOP	VERTICAL DOWEL	TIES	ADDITIONAL TIES AT TOP OF PIER			
P-1	30"x30"	(8) #7	(8) #7	#3@12" O.C.	(8) #3 AT 3" O.C.			
P•2	30"×24"	(8) #7	(8) #7	#3@12" O.C.	(8) #3 AT 3" O.C.			
P <u>-</u> 3	24"x30"	(8) #7	(8) #7	#3@12" O.C.	(8) #3 AT 3" O.C.			
P=4	29"×29"	(8) #7	(8) #7	#3@12" O.C.	(8) #3 AT 3" O.C.			
P <u>*</u> 5	30"x30"	(8) #7	(8) #7	#3@12" O.C.	(8) #3 AT 3" O.C.			
P=6	24"x30"	(8) #7	(8) #7	#3@12" O.C.	(8) #3 AT 3" O.C.			
P±7	24"x30"	(8) #7	(8) #7	#3@12" O.C.	(8) #3 AT 3" O.C.			









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**VAMC SALISBURY BUILDING 11-ADDITIONS** & RENOVATION

City, State Salisbury, NC

VA Contract Number VA246-12-C-0089

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Sheet Title TYPICAL BASE PLATE AND CONCRETE PIER DETAILS, **GRADE BEAM AND FOOTING SCHEDULES** 

August 28, 2014

Sheet Number

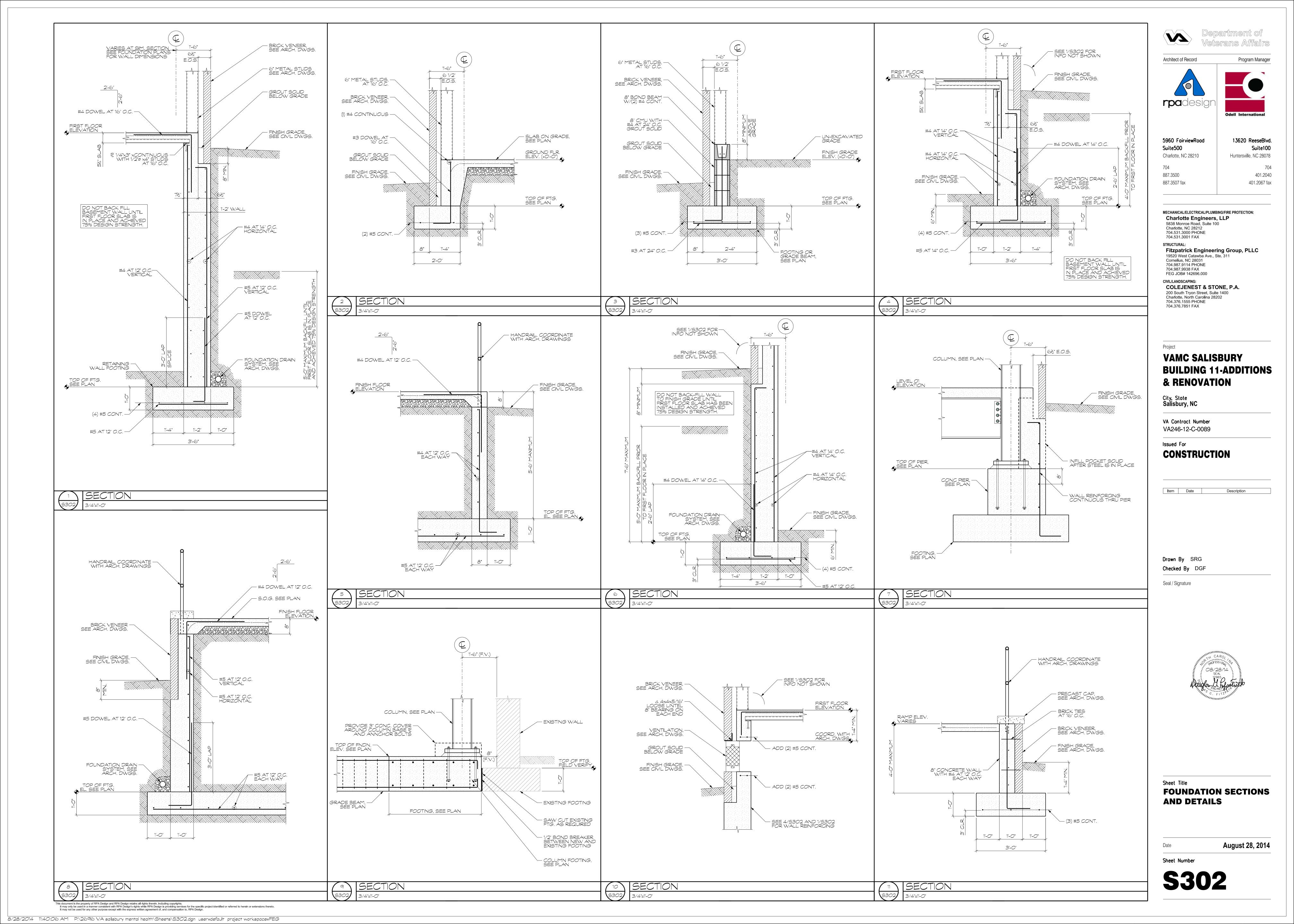
CAL CONCRETE PIER DETAILS AND SCHEDULE

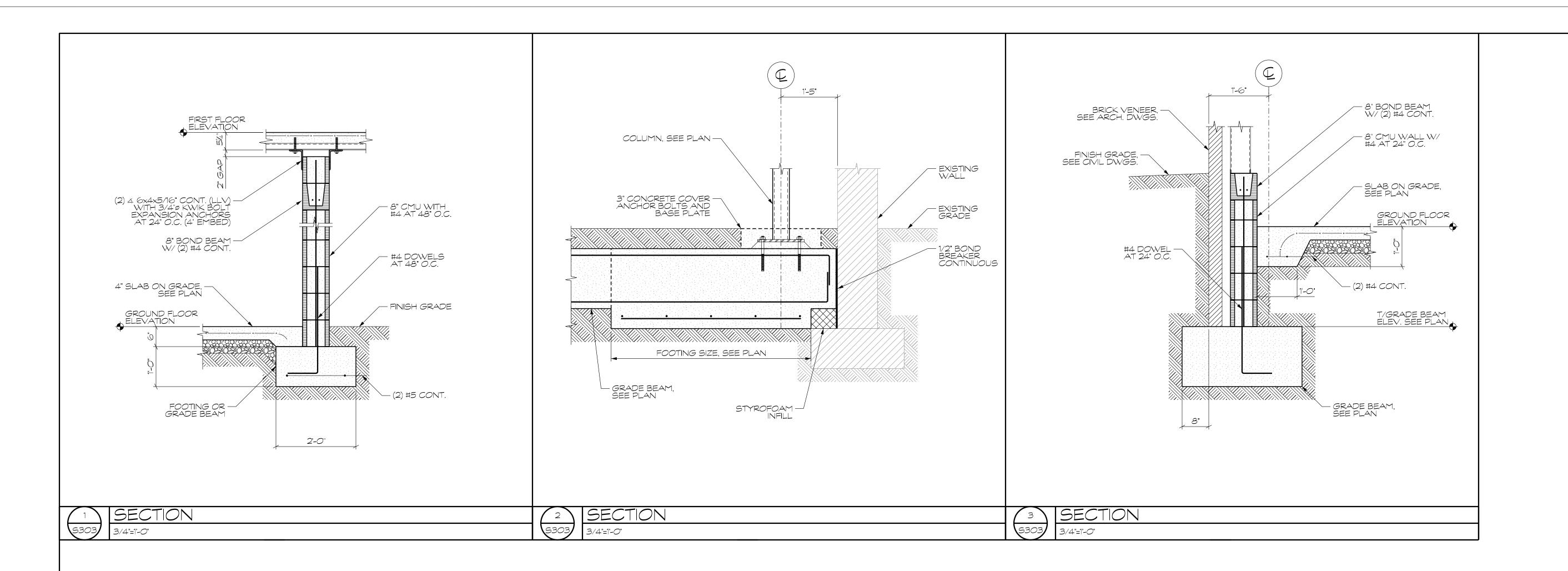
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City, State Salisbury, NC

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Item Date

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Sheet Title **FOUNDATION SECTIONS AND DETAILS** 

August 28, 2014

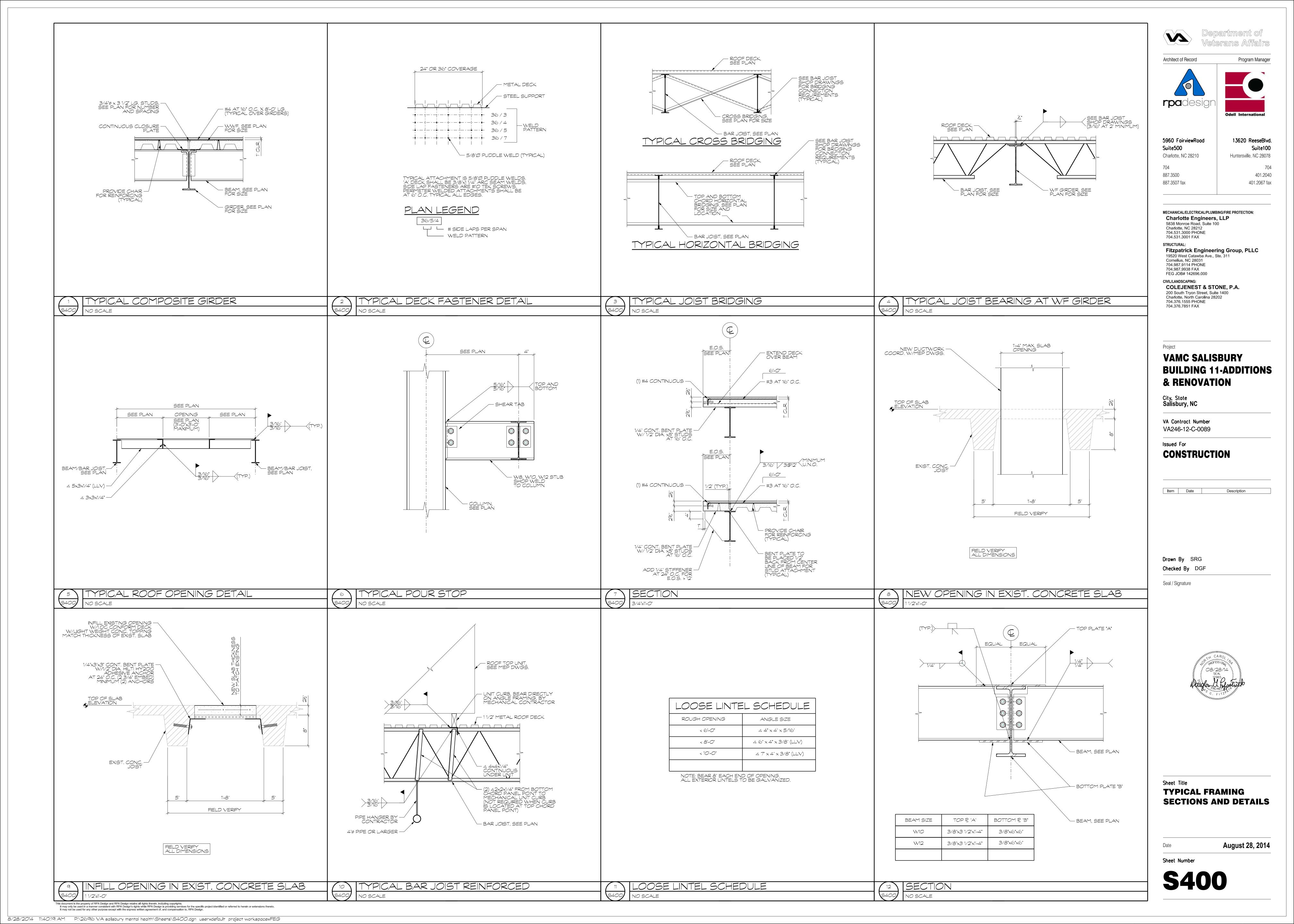
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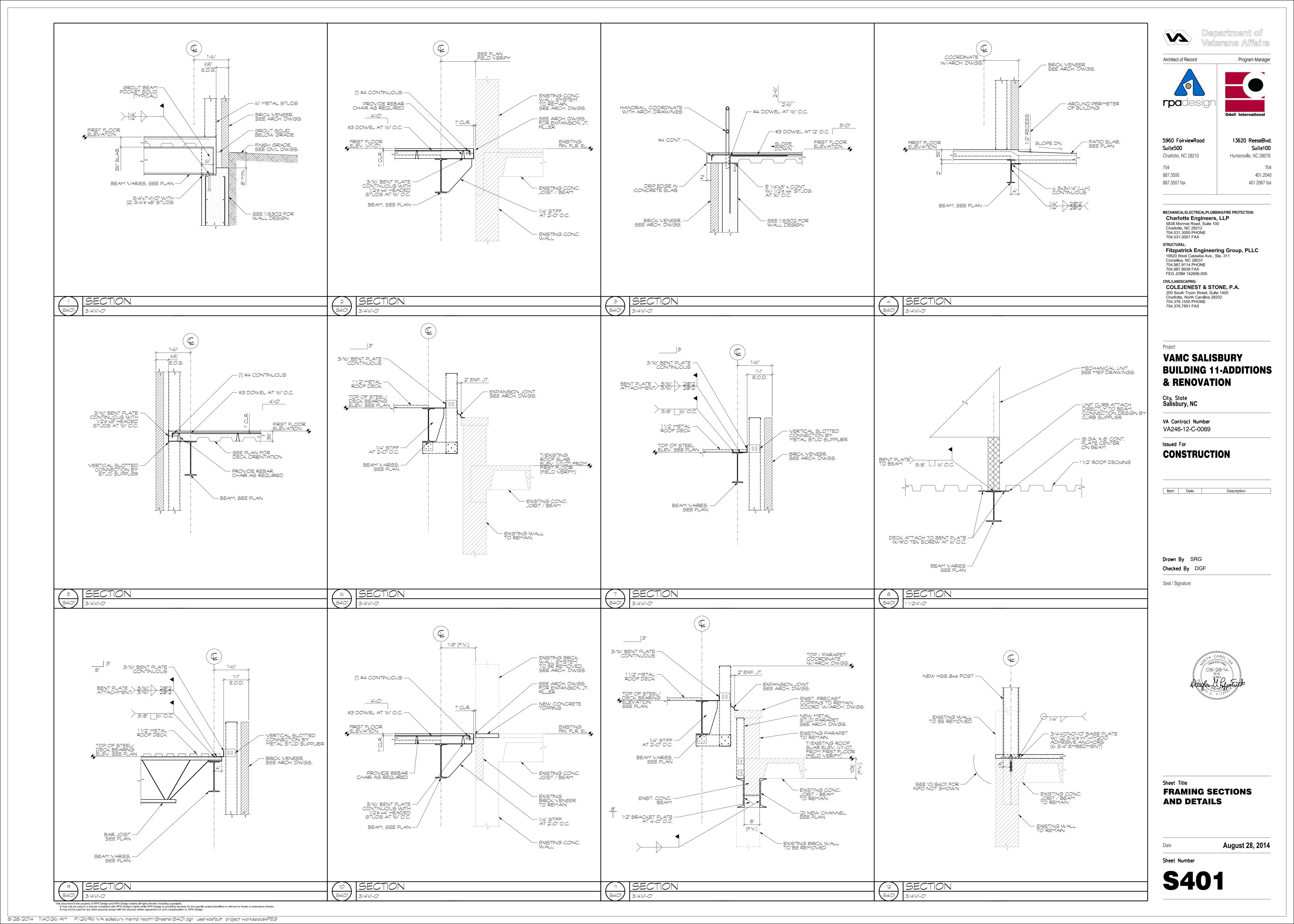
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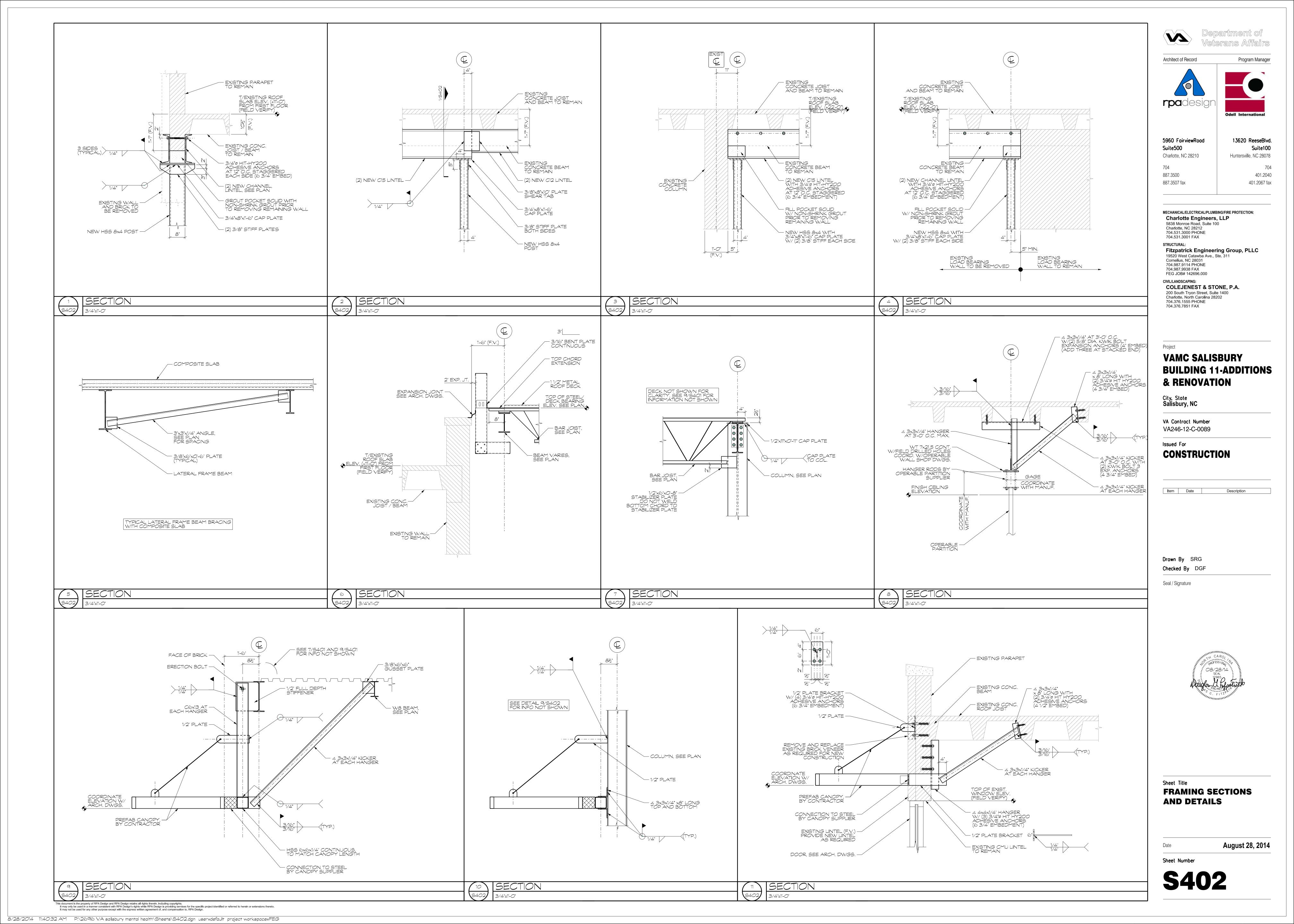
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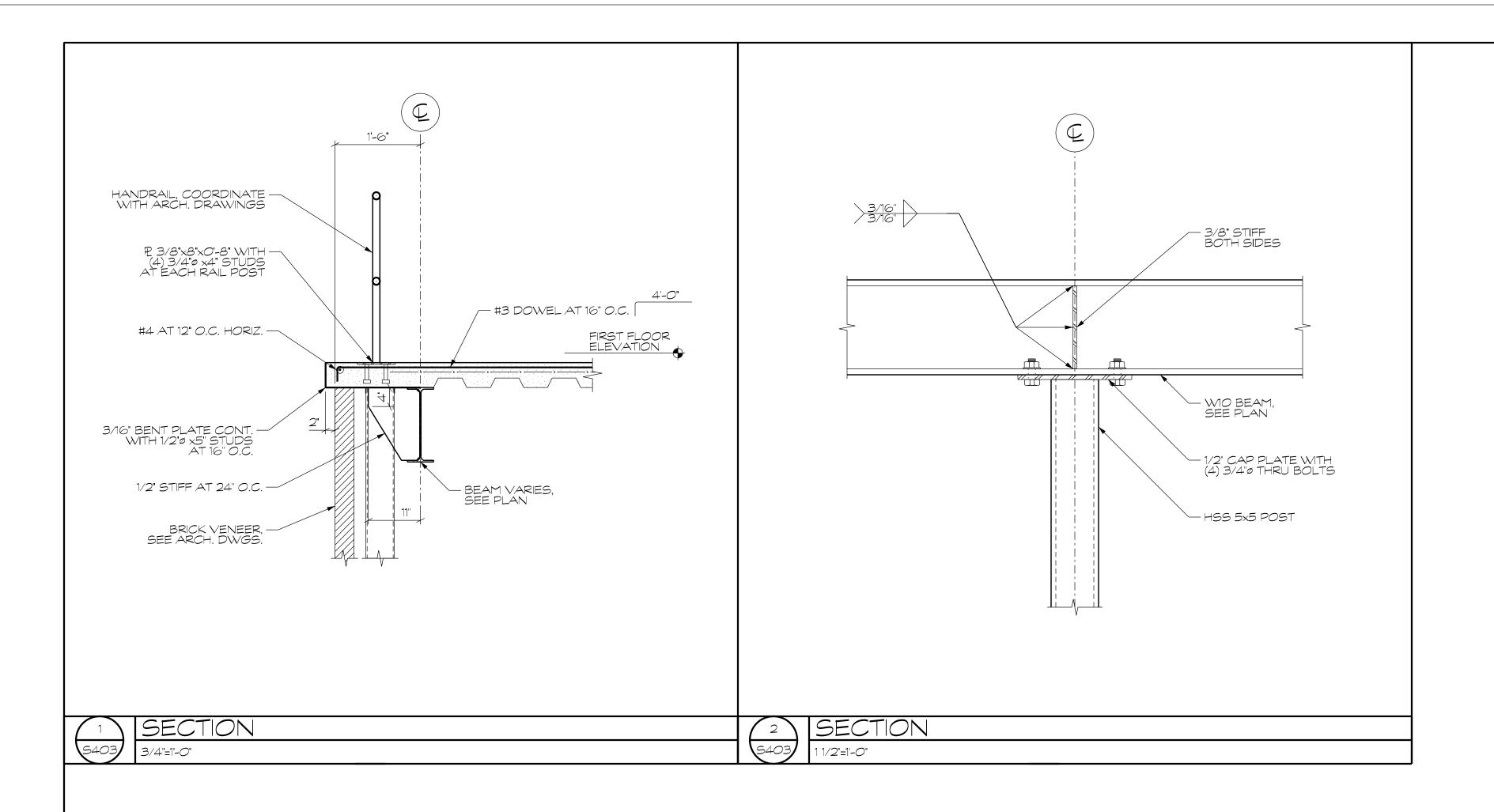
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City, State Salisbury, NC

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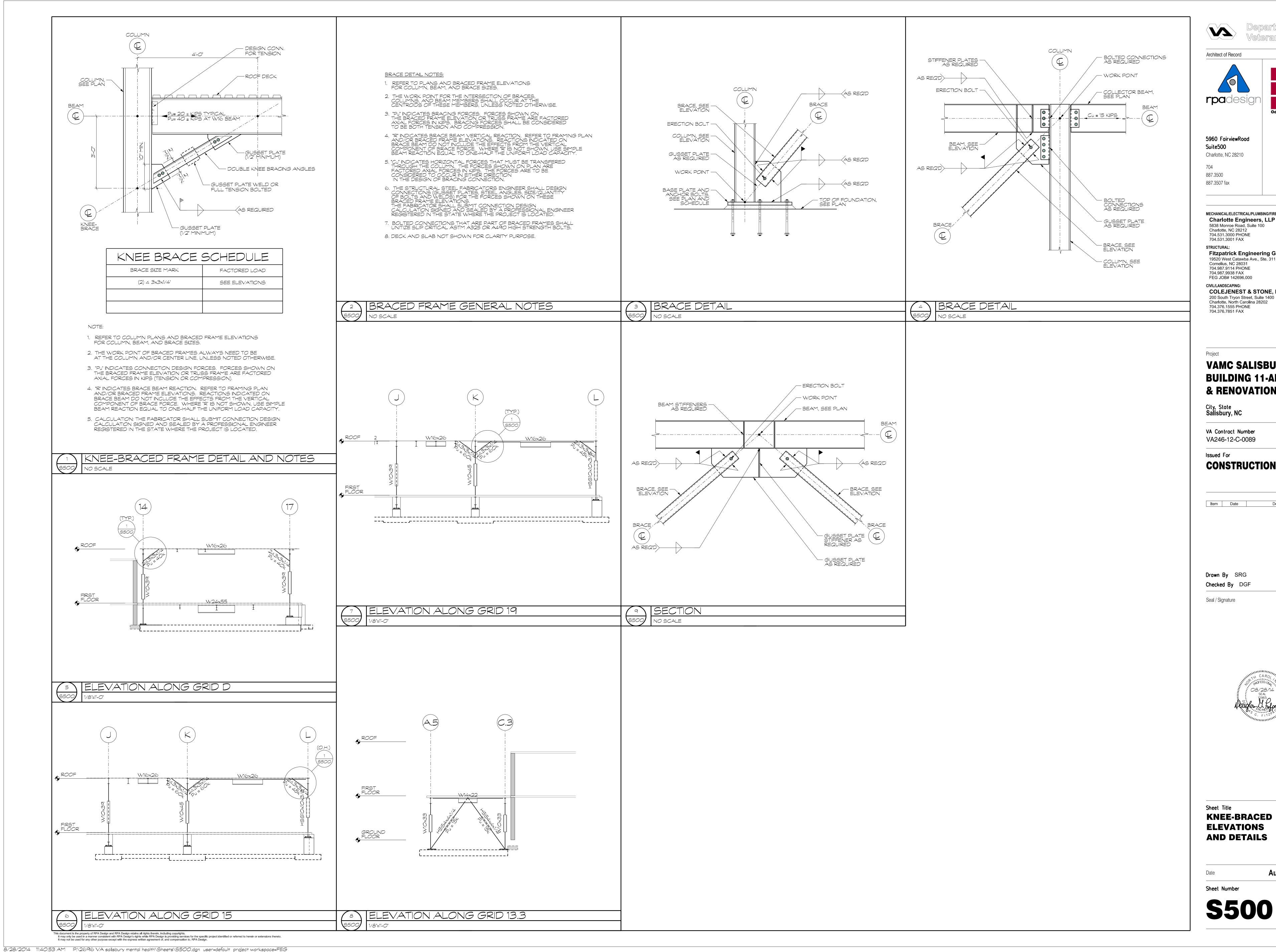


Sheet Title FRAMING SECTIONS **AND DETAILS** 

August 28, 2014

Sheet Number

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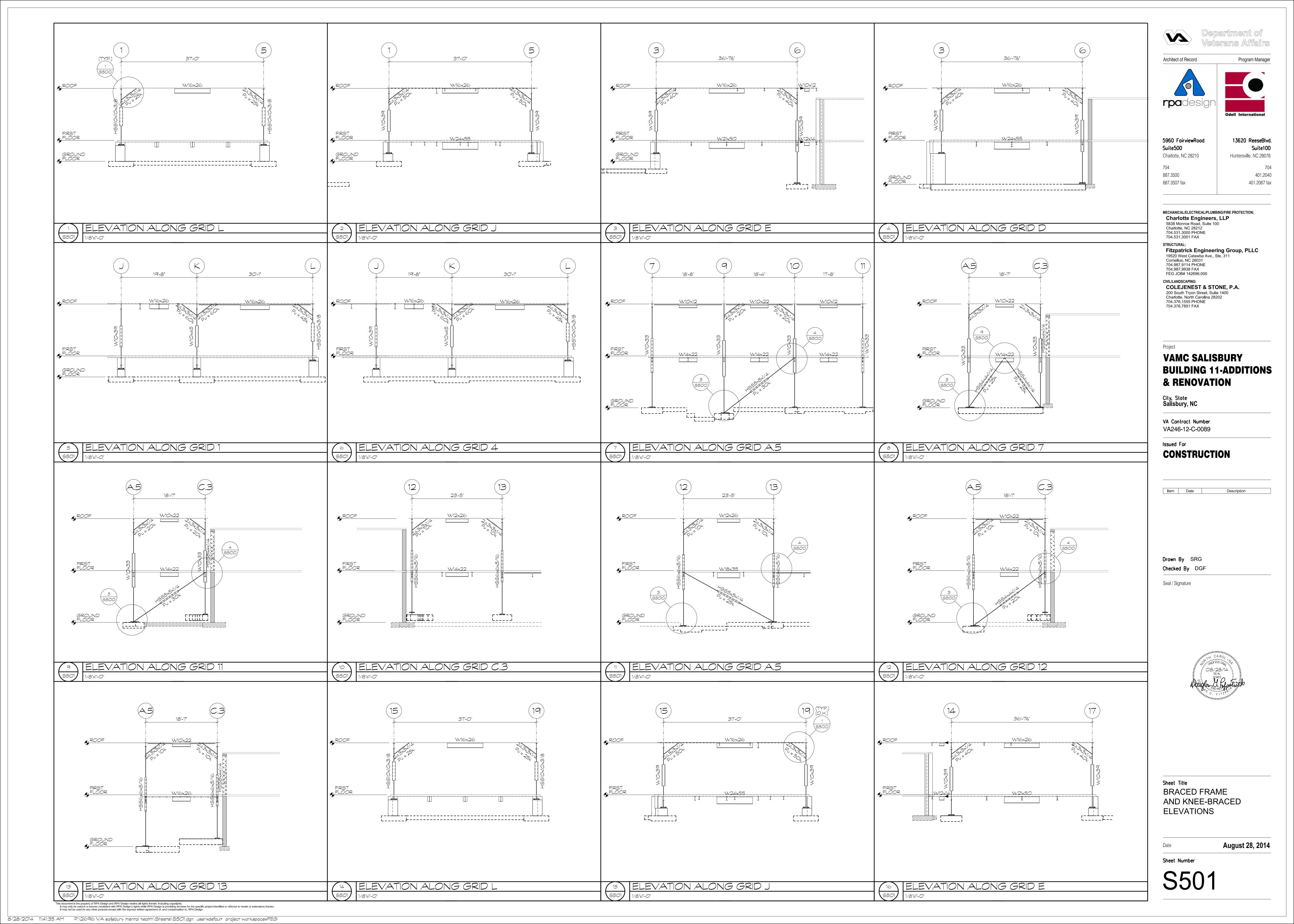
## CONSTRUCTION

Item Date Description

Sheet Title **KNEE-BRACED ELEVATIONS AND DETAILS** 

August 28, 2014

Sheet Number



STRUCTURAL STEEL CAST-IN-PLACE CONCRETE MASONRY 7 WOOD

SOILS AND FOUNDATIONS

☐ PILES

☐ PIERS SPRAY FIRE-RESISTANT MATERIAL MASTIC, INTUMESCENT COATINGS \_\_\_ EXTERIOR INSULATION AND FINISH SYSTEM SPECIAL CASES

SCHEDULE OF QUALIFICATIONS OF AGENCIES. INSPECTORS, AND TESTING TECHNICIANS CASE FORM 101

SPECIAL INSPECTION AGENCIES SHALL MEET THE FOLLOWING REQUIREMENTS:

1) INDEPENDENT - OBJECTIVE AND COMPETENT, DISCLOSE POSSIBLE CONFLICTS OF INTEREST. INTEREST.

2) EQUIPMENT - HAVE ADEQUATE EQUIPMENT, PERIODICALLY CALIBRATED TO PERFORM REQUIRED TESTS.

3) PERSONNEL - SHALL EMPLOY EXPERIENCED PERSONNEL EDUCATED TO CONDUCTING, SUPERVISING AND EVALUATING TESTS AND/OR INSPECTIONS

THE FOLLOWING KEY IS USED IN THE SCHEDULES OF SPECIAL INSPECTION TO IDENTIPY THE MINIMUM QUALIFICATIONS FOR THE INSPECTION AGENTS

THETTIMINU	TO QUALIFICATIONS FOR THE INSPECTION AGENTS
SE	STRUCTURAL ENGINEER - A LICENSED SE OR PE SPECIALIZING IN THE DESIGN OF BUILDING STRUCTURES AND REGISTERED IN THE STATE OF THE PROJECT
GE	GEOTECHNICAL ENGINEER - A LICENSED PE SPECIALIZING IN SOIL MECHANICS AND FOUNDATIONS AND REGISTERED IN THE STATE OF THE PROJECT
EIT	ENGINEER-IN-TRAINING - A GRADUATE ENGINEER WHO HAS PASSED THE FUNDAMENTALS OF ENGINEERING EXAMINATION.
ACI	AMERICAN CONCRETE INSTITUTE - GRADE 1 CERTIFIED CONCRETE FIELD TESTING TECHNICIAN.
CWI	AMERICAN WELDING SOCIETY - CERTIFIED WELDING INSPECTOR
SCWI	AMERICAN WELDING SOCIETY - SENIOR CERTIFIED WELDING INSPECTOR
CAWI	AMERICAN WELDING SOCIETY - CERTIFIED ASSOCIATE WELDING INSPECTOR
ASNT	AMERICAN SOCIETY OF NON-DESTRUCTIVE TESTING - LEVEL II OR III
SMSI	STRUCTURAL MASONRY SPECIAL INSPECTOR - CERTIFIED BY ICBO
SWSI	STRUCTURAL STEEL AND WELDING SPECIAL INSPECTOR
SFSI	SPRAY-APPLIED FIREPROOFING SPECIAL INSPECTOR
PCSI	PRESTRESSED CONCRETE SPECIAL INSPECTOR
RCSI	REINFORCED CONCRETE SPECIAL INSPECTOR

STRUCTURAL TESTS AND SPECIAL INSPECTIONS PER 2012 NORTH CAROLINA STATE BUILDING CODE (NCBC 1704)

SPECIAL INSPECTION GENERAL NOTES (NCBC 1704.1)

SPECIAL INSPECTIONS REQUIREMENT (NCBC 1704.1.2)

SPECIAL INSPECTIONS PER SECTION 1704 ARE REQUIRED FOR BUILDING, BUILDING COMPONENTS OR OTHER STRUCTURES PER THE FOLLOWING:

1. BUILDING OR OTHER STRUCTURES LISTED IN TABLE 1604.5 IN OCCUPANCY CATEGORY II IF:

1.1 BUILDING HEIGHT EXCEEDS 45 FEET (13.7 m) OR THREE STORIES, OR
1.2 THE BUILDING IS AN UNDERGROUND BUILDING PER SECTION 405.1

2. BUILDING OR OTHER STRUCTURES LISTED IN TABLE 1604.5

IN OCCUPANCY CATEGORIES III OR IV;

3. PILES, PIERS AND SPECIAL FOUNDATIONS PER SECTIONS 1704.8

THROUGH 1704.11, 1810.3.5.2.4 AND 1810.3.5.2.5;

4. RETAINING WALLS EXCEEDING 5 FEET (1424 mm) HEIGHT PER SECTION 1807.2;

5. SMOKE CONTROL AND SMOKE EXHAUST SYSTEMS;

6. SPRAYED FIRE-RESISTANT MATERIALS; OR

7. SPECIAL INGRECTIONS REPORT REQUIREMENT (NCBC 1704.1.3)

INSPECTION OF FABRICATORS (NCBC 1704.2)

SCHEDULE OF SPECIAL INSPECTIONS STRUCTURAL STEEL (NCBC 1704.3)

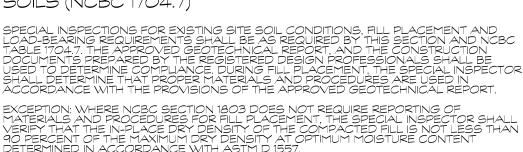
VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARDS	AGENT
MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS,     NUTS, AND WASHERS:     a. IDENTIFICATION MARKINGS TO CONFORM  ARKINGS T	PERIODIC	AISC 360, SECTION A3.3	
TO ASTM STANDARDS IN THESE DRAWINGS  b. MANUFACTURER'S CERTIFICATE OF  COMPLIANCE REQUIRED	PERIODIC	APPLICABLE ASTM MATERIAL STANDARDS	
INSPECTION OF HIGH-STRENGTH BOLTING:     a. BEARING TYPE CONNECTIONS	PERIODIC	AISC 360, SECTION M2.5 NCBC 1704.3.3	
b. SLIP-CRITICAL CONNECTIONS  1) TURN-OF-THE-NUT WITH MATCHMARKING, DIRECT TENSION INDICATOR, TWIST-OFF BOLT	PERIODIC	AISC 360, SECTION M2.5 NCBC 1704.3.3	
c. CALIBRATED WRENCH, TURN-OF-THE-NUT WITHOUT MATCHMARKING	CONTINUOUS	AISC 360, SECTION M2.5 NCBC 1704.3.3	SWSI
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL & DECK: a. IDENTIFICATION MARKINGS TO CONFORM TO AISC 360 b. OTHER STEEL WITH MARKINGS CONFORMING TO ASTM STANDARDS c. MANUFACTURERS CERTIFIED MILL TEST REPORTS REQUIRED	PERIODIC	ASTM A 6 OR ASTM A 568, ASIC 360, SECTION M5.5	
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS: a. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THESE DRAWINGS b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED		AISC 360, SECTION A3.5 AND APPLICABLE AWS A5 DOCUMENTS	
5. INSPECTION OF WELDING: a. STRUCTURAL STEEL 1) COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	CONTINUOUS	AWS D1.1 NCBC 1704.3.1	CWI
2) MULTI-PASS FILLET WELDS	CONTINUOUS	AWS D1.1 NCBC 1704.3.1	CWI
3) SINGLE-PASS FILLET WELDS > 5/16"	CONTINUOUS		CWI
4) PLUG AND SLOT WELDS	CONTINUOUS		CWI
5) SINGLE-PASS FILLER WELDS < 5/16" - SEE NOTE 1 BELOW	PERIODIC		CWI
6) FLOOR AND DECK WELDS - SEE NOTE 1 BELOW	PERIODIC	AWS D1.3	CWI
b. REINFORCING STEEL: NOT ALLOWED ON THIS PROJECT			
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH THESE DRAWINGS: a. DETAILS SUCH AS BRACING AND STIFFENING b. MEMBER LOCATIONS c. APPLICATION OF JOINT DETAILS AT EACH CONNECTION	PERIODIC	NCBC 1704.3.2	

DTES: 1. PROVIDE VISUAL INSPECTION OF ALL WELDS 2. SEE SCHEDULE OF QUALIFICATIONS OF SPECIAL INSPECTORS FOR DESIGNATION

SCHEDULE OF SPECIAL INSPECTIONS CAST IN DIACE CONCRETE (NICEC 1704 A)

VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARDS	AGEN <sup>-</sup>
I. INSPECTION OF REINFORCING STEEL, INCLUDING POST-TENSIONING TENDONS. VERIFY QUANTITY, SIZE AND PLACEMENT OF REINFORCEMENT	PERIODIC	NCBC 1913.4 ACI 318: 3.5 ACI 318: 7.1-7.7	
2. INSPECT REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5B.  NOT ALLOWED ON THIS PROJECT.			
3. INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	CONTINUOUS	NCBC 1911.5, 1912.1 ACI 318: 8.1.3, 21.2.8	
4. INSPECTION OF ANCHORS INTO HARDENED CONCRETE	PERIODIC	ACI 318 3.8.6, 8.1.3, 21.2.8 NCBC 1912.1	
5. VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	NCBC 1904.3, 1913.2, 1913.3 ACI 318: Ch. 4 ACI 318: Ch.5.2-5.4	ACI
6. SAMPLE FRESH CONCRETE, PERFORM SLUMP AND AIR CONTENT TESTS AND DETERMINE TEMPERATURE OF FRESH CONCRETE AT THE TIME OF MAKING SPECIMENS FOR STRENGTH TEST. SEE SPECIFICATIONS FOR FREQUENCY OF TESTING REQUIRED	CONTINUOUS	NCBC 1913.10 ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	ACI
7. MONITOR CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	NCBC 1913.6, 1913.7, 1913.8 ACI 318: 5.9, 5.10	ACI
8. MONITOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	NCBC 1913.9 ACI 318: 5.11-5.13	ACI
9. INSPECTION OF PRESTRESSED CONCRETE: A. APPLICATION OF PRESTRESSING FORCES. B. GROUTING OF BONDED PRESTRESSING TENDONS IN SEISMIC- FORCE-RESISTING SYSTEM.	CONTINUOUS CONTINUOUS	ACI 318: 18.20 ACI 318: 18.18.4	ACI
10. ERECTION OF PRECAST CONCRETE MEMBERS.	PERIODIC	ACI 318: Ch. 16	
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	PERIODIC	ACI 318: 6.2	ACI
12. INSPECT FORMWORK FOR SHAPE AND DIMENSIONS	PERIODIC	ACI 318: 6.1.1	

SCHEDULE OF SPECIAL INSPECTIONS



VERIFICATION AND INSPECTION OF SOILS	FREQUENCY	AGENT
VERIFY MATERIAL BELOW FOOTING TO ACHIEVE     BEARING CAPACITY	PERIODICALLY	GE
2. EXCAVATIONS EXTENDED TO PROPER DEPTH AND INTO PROPER MATERIAL	PERIODICALLY	GE
3. PERFORM CLASSIFICATIONS AND TESTING OF CONTROLLED FILL MATERIALS	PERIODICALLY	GE
4. VERIFY LIFT THICKNESS, MATERIAL AND DENSITIES AND COMPACTION	CONTINUOUS	GE
5. PRIOR TO PLACEMENT OF COMPACTED FILL OBSERVE SUB-GRADE IS PROPERLY PREPARED	PERIODICALLY	GE

SCHEDULE OF SPECIAL INSPECTIONS MASONRY (NCBC 1704.5.1, 1704.5.2)

LEVEL 1 SPECIAL INSPECTION

EMPIRICALLY DESIGNED, OCCUPANCY CATEGORY IV ENGINEERED MASONRY, OCCUPANCY CATEGORY I, II OR III

LEVEL I SPECIAL INSPECTION		REFERENCED STANDARDS			
VERIFICATION AND INSPECTION	FREQUENCY	NCBC SECTION	ACI 530 ASCE 5 TMS 402	ACI 530.1 ASCE 6 TMS 602	AGENT
1. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND APPROVED SUBMITTALS TO BE VERIFIED.	PERIODIC			ART. 1.5	SMSI
2. VERIFICATION OF fm AND fmc PRIOR TO CONSTRUCTION EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE.	PERIODIC			ART. 1.4B	SMSI
3. VERIFICATION OF SLUMP FLOW AND VSI AS DELIVERED TO THE SITE FOR SELF CONSOLIDATING GROUT	CONTINUOUS			ART. 1.5B.1.b.3	SMSI
4. AS MASONRY CONSTRUCTION BEGINS VERIPY THE FOLLOWING: a. PROPORTIONS OF SITE PREPARED MORTAR b. CONSTRUCTION OF MORTAR JOINTS c. LOCATION OF REINFORCEMENT, CONNECTORS, PRE-STRESS TENDONS AND ANCHORS d. PRESTRESSING TECHNIQUE e. GRADE AND SIZE OF PRESTRESSING TENDONS	PERIODIC PERIODIC PERIODIC PERIODIC PERIODIC			ART 2.6A  ART 3.3B  ART 3.4, 3.6A  ART 3.6B  ART 2.4B, 2.4H	SMSI
5. DURING CONSTRUCTION THE INSPECTION PROGRAM TO VERIFY: a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS. b. TYPE, SIZE AND LOCATION OF ANCHORS ALONG WITH OTHER DETAILS TO CONNECT TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION c. SIZE, GRADE AND TYPE OF REINF. ANCHOR BOLTS, TENDONS AND ANCHOR AGES. d. WELDING OF REINFORCING BARS e. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD AND HOT WEATHER (<40 OR >90 DEGREES F) f. APPLICATION AND MEASUREMENTS OF PRESTRESSING FORCE.	PERIODIC  PERIODIC  PERIODIC  CONTINUOUS PERIODIC  CONTINUOUS	SEC 2104.3 2104.4	SEC 1.2.2 (e), 1.16.1 SEC 1.15 SEC 2.1.9.7.2, 3.3.3.4(b)	ART 3.3F  ART 2.4, 3.4  ART 1.8C 1.8D  ART 3.6B	SMSI
6. PRIOR TO GROUTING VERIFY THE FOLLOWING: a. GROUT SPACE IS CLEAN b. PLACEMENT OF REINFORCEMENT c. PROPORTIONS OF SITE PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS d. CONSTRUCTION OF MORTAR JOINTS	PERIODIC PERIODIC PERIODIC PERIODIC		SEC 1.13	ART 3.2D ART 3.4 ART 2.6B ART 3.3B	SMSI
7. GROUT PLACEMENT SHALL BE VERIFIED a. GROUTING OF PRESTRESSED TENDONS	CONTINUOUS			ART 3.5 ART 3.6C	
8. PREPARATION OF ANY REQUIRED GROUT, MORTAR SPECIMENS SHALL BE OBSERVED	PERIODIC	SEC 2105.2.2 2105.3		ART 1.4	SMSI

SCHEDULE OF SPECIAL INSPECTIONS SPRAYED FIRE-RESISTANT MATERIALS (NCBC 1704.12)

3. APPLICATION: THE SUBSTRATE SHALL HAVE A MINIMUM AMBIENT TEMPERATURE BEFORE AND AFTER APPLICATION AS SPECIFIED IN THE WRITTEN INSTRUCTIONS OF APPROVED MANUFACTURERS. THE AREA FOR APPLICATION SHALL BE VENTILATED DURING AND AFTER APPLICATION AS REQUIRED BY THE WRITTEN INSTRUCTIONS OF APPROVED MANUFACTURERS.

THICKNESS: NO MORE THAN 10 PERCENT OF THE THICKNESS MEASUREMENTS OF THE SPRAYED FIRE-RESISTANT MATERIALS APPLIED TO FLOOR, ROOF AND WALL ASSEMBLIES AND STRUCTURAL MEMBERS SHALL BE LESS THAN THE THICKNESS REQUIRED BY THE APPROVED FIRE-RESISTANCE DESIGN, BUT IN NO CASE LESS THAN THE MINIMUM ALLOWABLE THICKNESS REQUIRED BY SECTION 1704.12.4.1.

b. FLOOR, ROOF AND WALL ASSEMBLIES: THE THICKNESS OF THE SPRAYED FIRE-RESISTANT MATERIAL APPLIED TO FLOOR, ROOF AND WALL ASSEMBLIES SHALL BE DETERMINED IN ACCORDANCE WITH ASTM E 605, MAKING NOT LESS THAN FOUR MEASUREMENTS FOR EACH 1,000 SQUARE FEET (93 M2) OF THE SPRAYED AREA IN EACH STORY OR PORTION THEREOF.

c. STRUCTURAL MEMBERS: THE THICKNESS OF THE SPRAYED FIRE-RESISTANT MATERIAL APPLIED TO STRUCTURAL MEMBERS SHALL BE DETERMINED IN ACCORDANCE WITH ASTM E 605. THICKNESS TESTING SHALL BE PERFORMED ON NOT LESS THAN 25 PERCENT OF THE STRUCTURAL MEMBERS ON EACH FLOOR. 1. BEAMS AND GIRDERS; AT BEAMS AND GIRDERS; THICKNESS MEASUREMENTS SHALL BE MADE AT NINE LOCATIONS AROUND THE BEAM OR GIRDER AT EACH END OF A 12-INCH (305 MM) LENGTH. 2. JOISTS AND TRUSSES: AT JOISTS AND TRUSSES, THICKNESS MEASUREMENTS SHALL BE MADE AT SEVEN LOCATIONS AROUND THE JOIST OR TRUSS AT EACH END OF A 12-INCH (305 MM) LENGTH.

3. WIDE-FLANGED COLUMNS: AT WIDE-FLANGED COLUMNS, THICKNESS MEASUREMENTS SHALL BE MADE AT 12 LOCATIONS AROUND THE COLUMN AT EACH END OF A 12-INCH (305 MM) LENGTH. 4. HOLLOW STRUCTURAL SECTION AND PIPE COLUMNS: AT HOLLOW STRUCTURAL SECTION AND PIPE COLUMNS, THICKNESS MEASUREMENTS SHALL BE MADE AT A MINIMUM OF FOUR LOCATIONS AROUND THE COLUMN AT EACH END OF A 12-INCH (305 MM) LENGTH.

5. DENSITY: THE DENSITY OF THE SPRAYED FIRE-RESISTANT MATERIAL SHALL NOT BE LESS THAN THE DENSITY SPECIFIED IN THE APPROVED FIRE-RESISTANCE DESIGN. DENSITY OF THE SPRAYED FIRE-RESISTANT MATERIAL SHALL BE DETERMINED IN ACCORDANCE WITH ASTM E 605. THE TEST SAMPLES FOR DETERMINING THE DENSITY OF THE SPRAYED FIRE-RESISTANT MATERIALS SHALL BE SELECTED AS FOLLOWS:

a. FLOOR, ROOF AND WALL ASSEMBLIES: THE TEST SAMPLES FOR DETERMINING THE COHESIVE/ADHESIVE BOND STRENGTH OF THE SPRAYED FIRE-RESISTANT MATERIALS SHALL BE SELECTED FROM EACH FLOOR, ROOF AND WALL ASSEMBLY AT THE RATE OF NOT LESS THAN ONE SAMPLE FOR EVERY 2,500 SQUARE FEET (232 M2) OF THE SPRAYED AREA IN EACH STORY OR PORTION THEREOF.

c. PRIMER, PAINT AND ENCAPSULANT BOND TESTS: BOND TESTS TO QUALIFY A PRIMER, PAINT OR ENCAPSULANT SHALL BE CONDUCTED WHEN THE SPRAYED FIRE-RESISTANT MATERIAL IS APPLIED TO A PRIMED, PAINTED OR ENCAPSULATED SURFACE FOR WHICH ACCEPTABLE BOND-STRENGTH PERFORMANCE BETWEEN THESE COATINGS AND THE FIRE-RESISTANT MATERIAL HAS NOT BEEN DETERMINED. A BONDING AGENT APPROVED BY THE SFRM MANUFACTURER SHALL BE APPLIED TO A PRIMED, PAINTED OR ENCAPSULATED SURFACE WHERE THE BOND STRENGTHS ARE FOUND TO BE LESS THAN REQUIRED VALUES.

SCHEDULE OF SPECIAL INSPECTIONS EXTERIOR INSULATION AND FINISHING SYSTEMS (EIFS) (NCBC 1704.14)

SPECIAL INSPECTIONS SHALL BE REQUIRED FOR ALL EIFS APPLICATIONS EXCEPTIONS:

1. SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR EIFS APPLICATIONS INSTALLED OVER A WATER-RESISTIVE BARRIER WITH A MEANS OF DRAINING MOISTURE TO THE EXTERIOR.

2. SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR EIFS APPLICATIONS INSTALLED OVER MASONRY OR CONCRETE WALLS. WATER-RESISTIVE BARRIER COATING COMPLYING WITH ASTM E 2570 REQUIRES SPECIAL INSPECTION OF THE WATER-RESISTIVE BARRIER COATING WHEN INSTALLED OVER A SHEATHING SUBSTRATE.

SCHEDULE OF SPECIAL INSPECTIONS RETAINING WALLS (NCBC 1807.2)

THE APPROVED SOILS REPORT, REQUIRED BY NCBC SECTION 1803.2, SHALL BE USED TO DETERMINE COMPLIANCE VERIFY COMPLIANCE WITH INSPECTION CHART

VERIFICATION AND INSPECTION	FREQUENCY	AGENT
VERIFY SUPPORT SYSTEM ADEQUATE FOR     INTENDED SITE CONDITIONS	PERIODICALLY	GE
2. VERIFY QUALITY OF CONSTRUCTION MATERIALS FOR CONFORMANCE WITH SPECIFICATIONS.	PERIODICALLY	GE
3. VERIFY ACTUAL SOILS CONDITIONS MEET THOSE INTENDED IN DESIGN.	PERIODICALLY	GE
4. VERIFY BACK-FILL MATERIALS AND DRAINAGE SYSTEMS FOR COMPLIANCE WITH PLANS AND SPECIFICATIONS.	PERIODICALLY	GE

STATEMENT OF SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE (IBC REFERENCE 1705.3)

□ REQUIRED

■ NOT REQUIRED Y N SEISMIC-FORCE-RESISTING SYSTEMS IN STRUCTURES ASSIGNED TO SEISMIC ☐ ■ DESIGN CATEGORY C, D, E OR F AND SPECIFICALLY DETAILED FOR SEISMIC

Y N DESIGNATED SEISMIC SYSTEMS IN STRUCTURES ASSIGNED TO SEISMIC ☐ ■ DESIGN CATEGORY D, E OR F.

EXCEPTIONS:

1. LIGHT WOOD FRAMING OR LIGHT GAUGE COLD-FORMED STEEL FRAMING, Sds < 0.5 AND BUILDING HEIGHT <35 FEET ABOVE GRADE

2. REINFORCED MASONRY OR REINFORCED CONCRETE BUILDINGS WITH Sds < 0.5 AND BUILDING HEIGHT <25 FEET ABOVE GRADE

3. DETACHED ONE- OR TWO-FAMILY DWELLINGS NOT EXCEEDING TWO STORIES IN HEIGHT PROVIDED PLAN DOES NOT HAVE PLAN OR VERTICAL IRREGULARITIES PER SECTION 12.3.2 OF ASCE 7:

1. TORSIONAL IRREGULARITY

2. NONPARALLEL SYSTEMS

3. STIFFNESS IRREGULARITY - EXTREME SOFT/SOFT STORY

4. WEAK STORY

SEISMIC-FORCE-RESISTING SYSTEM:

STRUCTURAL STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

STATEMENT OF SPECIAL INSPECTIONS FOR WIND RESISTANCE (IBC REFERENCE 1705.4)

☐ REQUIRED ■ NOT REQUIRED

REQUIRED FOR ALL STRUCTURES CONSTRUCTED IN THE FOLLOWING AREAS:

1. WIND EXPOSURE CATEGORY B, WHERE 3-SECOND-GUST BASIC

WIND SPEED IS 120 MPH OR GREATER

2. WIND EXPOSURE CATEGORIES C AND D, WHERE 3-SECOND-GUST BASIC

WIND SPEED IS 110 MPH OR GREATER.

DETAILED REQUIREMENTS. THE FOLLOWING ITEMS HAVE BEEN INCORPORATED INTO THE PLANS AND DETAILS FOR THIS PROJECT:

1. ROOF CLADDING AND ROOF FRAMING CONNECTIONS
2. WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING
3. ROOF AND FLOOR DIAPHRAGM SYSTEMS, INCLUDING COLLECTORS, DRAG STRUTS, AND BOUNDARY ELEMENTS. SEE PLANS FOR DECKING ATTACHMENTS AT SUPPORTS AND SIDELAPS
4. VERTICAL WIND-FORCE-RESISTING SYSTEMS, INCLUDING BRACE FRAMES, MOMENT FRAMES AND SHEARWALLS
5. WIND-FORCE-RESISTING SYSTEM CONNECTIONS TO FOUNDATION
6. FABRICATION AND INSTALLATION OF COMPONENTS OR ASSEMBLIES REQUIRED TO MEET THE IMPACT RESISTANCE REQUIREMENTS OF IBC SECTION 1609,1.2

MAIN WIND-FORCE-RESISTING SYSTEM: STRUCTURAL STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

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**VAMC SALISBURY BUILDING 11-ADDITIONS** & RENOVATION

City, State Salisbury, NC

VA Contract Number VA246-12-C-0089

Issued For

CONSTRUCTION

Item Date Description

Checked By DGF

Seal / Signature



Sheet Title **SPECIAL INSPECTION NOTES** 

August 28, 2014

Sheet Number

SPECIAL INSPECTION NOTES

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